

## Ain Shams University Faculty of Engineering Department of Structural Engineering

# **Behavior of Piled Raft Foundation in Calcareous Cemented Sand**

#### A Thesis

Submitted in Partial Fulfillment for the Requirements of the Degree of Doctor of Philosophy in Civil Engineering (Structural Engineering)

#### Submitted by

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#### STATEMENT

This dissertation is submitted to Ain Shams University for the degree of Doctor of Philosophy in Civil Engineering (Structural Eng.)

The work included in this thesis was carried out by the author in the Structural Engineering Department, Faculty of Engineering, Ain Shams University, Cairo, Egypt.

No part of this thesis has been submitted for a degree or qualification at any other university or institution.

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### **Ain Shams University Faculty of Engineering**



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#### **ABSTRACT**

The calcareous cemented sand exists in many places around the world where in last two decades there was a huge development. During such huge development, the foundation design was a major challenge. This research aims to introduce the piled raft foundation as a competitor solution for this challenge.

To achieve this goal, a research plan consist of four stage was planned by the author. These stages are investigating the characteristic behavior of calcareous cemented sand, selecting a presentable constitutive law for the calcareous cemented sand, utilizing pile load test result to calibrate the constitutive law parameters, and analysis a case history dealing with conventional piled foundation using the piled raft concept respectively.

Investigating the characteristic behavior of calcareous cemented sand reveals a cohesion intercept because of existence of carbonates as bonding agent at points of contact between the particles and interlocking between the particles. Also, investigating the characteristic behavior of calcareous cemented sand shows increased stiffness with depth, small difference between the peak friction angle and residual friction angel, and unloading reloading stiffness is higher than the loading stiffness.

Based on this characteristic behavior of calcareous cemented sand, double hardening soil model has been utilized as a suitable constitutive law to represent the stress strain behavior of calcareous cemented sand. In light of available published triaxial tests on calcareous cemented sand, the parameters of double

hardening soil model were estimated. Moreover, calibration of these parameters was conducted via back calculation of real pile load tests in calcareous cemented sand.

In the end of this research, a conventional case history dealing with pile foundation was redesigned applying the piled raft concept, where raft, pile, and soil mutual interactions were introduced using a 3D finite elements analysis. The soil was modeled by the double hardening model, the raft was utilized using plate element, and the piles were modeled using embedded pile model.

Keywords: Calcareous cemented sand, hardening soil model, single pile, piled raft.

#### **GLOSSARY OF SYMBOLS**

B : Pore water pressure coefficient

c : Cohesion

c<sub>u</sub>: Coefficient of Uniformity

 $C_N^*$ : Correction Factor of Standard penetration Test

 $D_{10}$  : size of 10 % passing in sieve analysis

D<sub>r</sub> : Relative Density

E : Young's modulus

E<sub>50</sub> : Young's modulus at stress equal to 50% of ultimate stress

E<sub>u</sub> : Young's modulus for triaxial unloading condition

E<sub>oed</sub>: Young's modulus for oedometer

Eb : Base Stiffness (modulus of soil below the base)

fb : Ultimate Base Bearing

fs : Ultimate Shaft Friction

G: Shear modulus of the soil

 $K_0^{nc}$ : The value of  $K_0$  in primary one-dimensional compression

 $K_{0(u)}$ : at rest earth pressure coefficient during initial unloading

L : Length of the pile

LL: Liquid Limit

Ms : Shaft Flexibility factor

m: The power of the stress-dependent stiffness formulation

PL: Plastic Limit

P<sub>ref</sub>: Reference stress level

Q : Applied load

R<sub>f</sub>: Failure ratio, which determines the strain level at failure

 $r_0$ : Pile radius

 $r_{m}$  : Influence radius at which shear stresses become negligible

u : Pore water Pressure

 $\Delta \phi_{\rm l}$  : Correction for the particle shape

 $\Delta \phi_2$ : Correction for the particle size (effective size,  $D_{10}$ )

 $\Delta \phi_3$ : Correction for gradation (uniformity coefficient,  $c_u$ )

 $\Delta \phi_4$ : Correction for relative density (D<sub>r</sub>)

 $\Delta \phi_5$  : Correction for type of mineral

v: Poisson's ratio

 $v_{ur}$ : Poisson's ratio for unloading and reloading

 $\phi$ : Friction angle

 $\phi_{\text{max}}$ : Peak friction angle

 $\phi_{cv}$ : Critical state friction angle

 $\psi$  : Dilatancy angle

 $\sigma_3$ : Confining Pressure of the Triaxial Cell

 $\sigma_n$ : Total Normal Stresses

 $\sigma_n$ : Effective Normal Stresses

 $\sigma'v$ : Effective Overburden Stresses

#### TABLE OF CONTENTES

Chapter (1): INTRODUCTION	1
1.1 Statement of the Problem	1
1.2 Research Objectives	3
1.3 Thesis Outline	4
CHAPTER (2): PROPERTIES OF THE CALCAREOUS CE SAND IN KUWAIT	
2.1 Introduction	5
2.2 Geological Formation of Calcareous Sand in Kuwait	5
2.3 Soil Classification According to Carbonate Content	9
2.4 Mineral Component of the Calcareous Cemented Sand	9
2.5 Kuwaiti Calcareous Sand and Grain Size Distribution	11
2.6 Cementation of Calcareous Sand in Kuwait	12
2.7 Exploration and Sampling of Calcareous Cemented Sand	14
2.8 Interpretation of Laboratory and Field Test Results of Cemented	I Sand18
CHAPTER (3): STRENGTH CHARACTERS OF CALC	AREOUS
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM	AREOUS 1 FIELD
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS	AREOUS // FIELD20
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS	<b>AREOUS 1 FIELD</b> 20
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS	<b>AREOUS 1 FIELD</b> 2020
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS  3.1 Introduction 3.2 Cone Penetration Test (CPT) 3.3 Standard Penetration Test (SPT)	AREOUS // FIELD202020
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS  3.1 Introduction 3.2 Cone Penetration Test (CPT) 3.3 Standard Penetration Test (SPT) 3.4 Pressuremeter Test	AREOUS // FIELD202020
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS  3.1 Introduction 3.2 Cone Penetration Test (CPT) 3.3 Standard Penetration Test (SPT)	AREOUS // FIELD202020
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS  3.1 Introduction 3.2 Cone Penetration Test (CPT) 3.3 Standard Penetration Test (SPT) 3.4 Pressuremeter Test	AREOUS // FIELD
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS.  3.1 Introduction	AREOUS // FIELD
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS.  3.1 Introduction	AREOUS // FIELD
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS  3.1 Introduction 3.2 Cone Penetration Test (CPT) 3.3 Standard Penetration Test (SPT) 3.4 Pressuremeter Test 3.5 Consolidation Testing in Calcareous Cemented Sand 3.6 Direct Shear Testing in Calcareous Cemented Sand 3.7 Triaxial Shear Testing in Calcareous Cemented Sand CHAPTER (4): ASPECTS OF MODELLING THE CALC	AREOUS 1 FIELD
CHAPTER (3): STRENGTH CHARACTERS OF CALC CEMENTED SAND IN KUWAIT FROM AND LABORATORY TESTS	AREOUS 1 FIELD

4.3 Soil Stiffness	41
4.4 Irreversible Deformation	
4.6 Possibilities and Limitation of Some Existing Models for	72
Geotechnical Applications	48
4.6.1 Elastic models (Hooke's Low and Duncan -Chang model)	49
4.6.2 Elasto-plastic models	51
4.6.2.1 Mohr-Coulomb model	
4.6.2.2 Drucker-Prager	
4.7 Numerical Modeling	
4.8 Selection of Mohr Coulomb & Hardening Soil Model Parameters for Calcareous Cemented Sand	
4.9 Quantification of Model Parameters	
4.10 Identification of Basic and Advanced Model Parameters	
4.11 Obtaining of Preliminary Values of Parameters of	
Double Hardening Model	59
4.11.1 Friction angle	
4.11.2 Cohesion intercept	
4.11.3 Modulus of elasticity	
parameters	62
4.12 Preliminary Values of Double Hardening Model Parameters	
for Calcareous Cemented Sand	65
4.13 Verification of the Response of the Calcareous Sand by Using the	
Double Hardening Soil Model	65
CHAPTER (5): PREDICTING SETTLEMENT OF SINGLE IN CALCAREOUS CEMENTED SAND	
5.1 Predicting Settlement of Single Pile	
5.2 Empirical Approach	
5.3 Load Transfer Approach Coyle and Reese (1966)	
5.4 Methods Based on Theory of Elasticity	
5.5 Randolph and Worth (1978)	
5 6 Fleming (1992)	73

5.7 Pile Load Test in Gulf Region	74
5.8 Design Values of Skin Friction and End Bearing of Bored Piles	
Calcareous Cemented Sand	75
5.9Numerical Analysis of Single Pile Performance in the	
Calcareous Cemented Sand	81
5.9.1 Performance of volume Pile	81
5.9.2 Results of embedded Pile	85
5.9.3 Comparison between volume pile and embedded pile at	
working loads	87
CHAPTER (6): PILED RAFT FOUNDATION IN CALCAREOUS CEMENTED SAND (KUV SOIL)	VAITI
6.1 Introduction	89
6.2 Piled Raft in Gulf Region	90
6.3 Examples of High Rise Towers (Salmiya Complex)	91
6.4 Calcareous Cemented Sand and Kuwaiti Subsoil	95
6.5 Analysis Methods of the Piled Raft	95
6.5.1 Approximate method	96
6.5.2 Boundary element method	96
6.5.3 Finite element method	96
6.5.4 Combined boundary element and finite element method	98
6.6 Numerical Model of Salmiya Complex by using Double	
Hardening Model	98
6.7 Soil Parameters for Salmiya Tower	98
6.8 3D Finite element Model	101
6.9 Analysis of Piled Raft	103
6.10 Results of the Settlement of Salmiya Complex	104
6.11 Contact Pressure and Stress Distribution under the Piled Raft of	
Salmiya Complex	111
6.12 Tilting of Salmiya Complex	113
6.13 Results of the Piles at Salmiya Complex	113

CHAPTER (7): SUMMARY, CONCLUSIONS, AND	
NDATION OF FUTURE RESAECHI	<b>ES</b> 125
7.1 Introduction	125
7.2 Summary and Conclusions	125
REFERENCES	128
Appendix	139

#### LIST OF FIGURE

Figure 2.1: Map of Kuwait
(from http://en.wikipedia.org/wiki/File:Ku-map.gif)6
Figure 2.2: Stratigraphic sequence of Kuwait and South of Iran
(after Fuchs, 1968)7
Figure 2.3: Location of Sabkha soil in Kuwait and in Gulf
Region (after Ismael, 1993)8
Figure 2.4: Grain size analysis of calcareous and clean sand
(after Ismael, 1993)
Figure 2.5: Types of inter-particles contact and main sources of
cohesion in cohesive sand
Figure 2.6: Multi-stage excavation in cemented sand in Kuwait
Figure 2.7: Multi-Stage Excavation using steep slope in the cemented
sand Layer16
Figure 2.8: Schematic arrangement of the Denison Sampler
(after Boulanger, 2009)
Figure 2.9: Pitcher Barrel sampler (after Boulanger, 2009)
Figure 3.1: Soil profile at the test sites after (Ismael et al., 1988)
Figure 3.2: Comparison between the SPT correction factors for
calcareous sand with the proposed formula for clean silica
sand (after Ismael et. al, 1988)
Figure 3.3: Comparison between test results and Skempton
(1986) correlation for fine sand (after Ismael et. al, 1988)
Figure 3.4: Location plan showing test corridor and footing test sites
(after Ismael et. al, 1986)
Figure 3.5: Penetration test results for the test sites
(after Ismael et. al, 1986)
Figure 3.6: Soil condition at the site of the Antenna tower
(after Al Sanad and Ismael, 1993)27
Figure 3.7: Pressuremeter moduli with depth and range of moduli
earned from plate load tests at the Antenna tower site