Ain Shams University Faculty of Engineering

# ANALYSIS OF SHEAR WALLS IN FRAMED STRUCTURES

Thesis Submitted For The M.Sc.Degree In Structural Engineering

By
Faten Ahmed Shaker
B.Sc. 1983 (Structural Engineering)

1)4.1773 F. A

Under The Supervision Of
Dr.Mostafa Kamel Metwally Zidan
Professor Of Theory Of Structures
Faculty of Engineering, Ain Shams University







### STATEMENT

This thesis is submitted to Ain Shams University for the degree of Master of Science in Structural Engineering.

No part of this thesis has been previously submitted for a degree or a qualifacation.

Date

Signature:

Name : Faten Ahmed Shaker

#### APPROVAL SHEET

#### Analysis of Shear Walls in Framed Structures by Faten Ahmed Shaker

Approved by:

Prof.Dr. Abdel Rahman Sadek Bazaraa

Head of Structural Department Cairo University Bozariae M. G. Aleshol

Prof.Dr. Mahmoud Galal Hashish

Professor of Strucural Engineering Ain Shams University

Prof.Dr. Mostafa Kamel Metwally Zidan.

Professor of Structural Engineering Ain Shams University Tih.

Date

Comittee in charge

#### Acknowledgments

The author wishes to express her sincere appreciation and deepest gratitude to her research supervisor Prof.Dr. Mostafa Kamel Melwaly Zidan, Professor of Theory of Structures, Faculty of Engineering, Ain Shams University, for his inestimable guidance, valuable criticism, and constant encouragement throughout the course of this research work.

#### Ain Shams University

Faculty of Engineering Structural Department

## Abstract for the M.Sc.Thesis Submitted by Eng. Faten Ahmed Shaker

Title of the Thesis: Analysis of Shear Walls in Framed Structures

Supervisors : Prof.Dr.Mostafa Kamel Metwally Zidan

Registration Date: 12-11-1984

**Examination Date:** 

#### Abstract

The thesis deals with the analysis of shear walls with openings subjected to lateral loads. The finite element method is used for the analysis as it is a powerful tool of illustrating the internal action in all wall elements as well as the details of wall deformation efficiently.

A comprehensive finite element computer package is used for the analysis, the package includes the elements necessary to accurately simulate the shear wall whatever is its geometry. Both symmetrical and unsymmetrical curtailed shear walls with openings are investigated.

A comprehensive parametric study on these walls is performed concerning many parameters such as: shear wall breadth, opening width, soil modulus of elasticity, curtailment length, and raft foundation thickness. The results indicating the wall drift, the distribution of stresses under the wall foundation and the internal forces in lintel beams are drawn. The results of the study are analysed, discussed and useful recommendations regarding the analysis and design of curtailed shear walls with openings are stated.

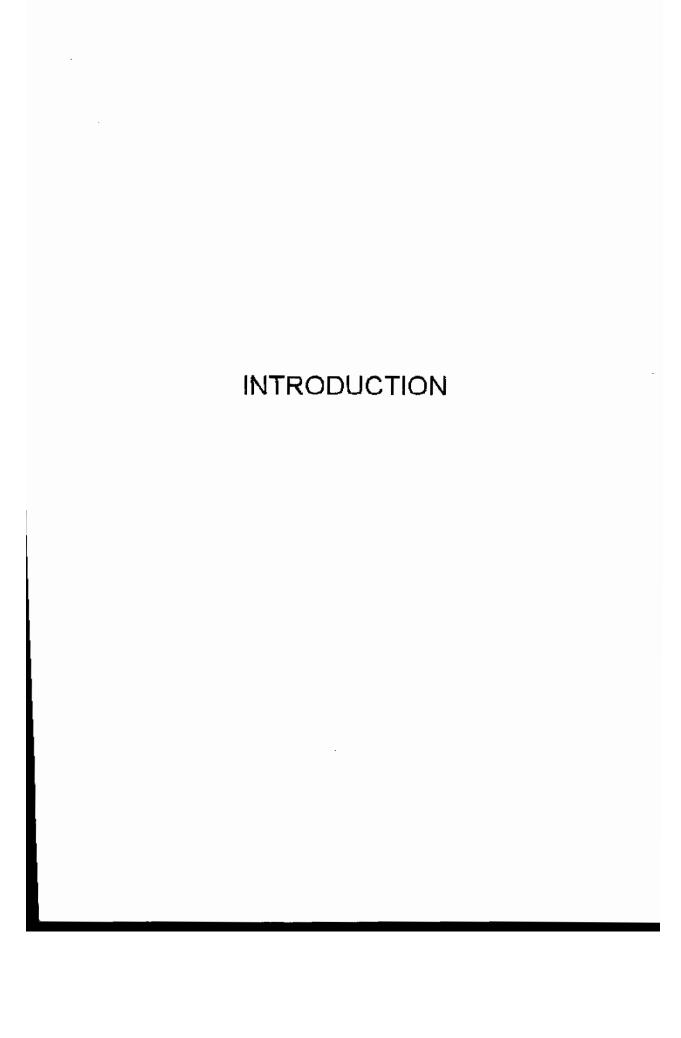
#### Table of Contents

| ACKNOWLEDGMENTi                                |
|--|
| ABSTRACTii                                     |
| Table of Contentsiii                           |
|  |
| INTRODUCTION1                                  |
|  |
| Chapter 1: REVIEW OF PREVIOUS WORK6            |
| 1.1. INTRODUCTION6                             |
| 1.2. REVIEW OF PREVIOUS INVESTIGATIONS7        |
| 1.3. Practical Example71                       |
| 1.3.1. Different Methods of Analysis74         |
| 1.3.1.1. The Continuum Approach Method74       |
| 1.3.1.2. The Frame Idealization Method76       |
| 1.3.1.3. The Finite Elements Method76          |
| 1.3.2. Tables Representing The Results77       |
| 1.3.2.1. Deflections of the Shear Wall         |
| in cms77                                       |
| 1.3.2.2. Shearing Forces of the Beams in ton78 |
| 1.3.2.3. Bending Moments of the Beams in mt79  |
| 1.3.2.4. Normal Force of the Shear Wall        |
| in tons80                                      |
| 1.3.3. Results81                               |

| Chapter | 2: COMPUTATIONAL ANALYSIS FOR SHEAR WALLS82  |  |  |  |
|---------|--|--|--|--|
| 2.1.    | Introduction82   |  |  |  |
| 2.2.    | The adopted computer program83   |  |  |  |
| 2.3.    | Types of elements87  |  |  |  |
|         | 2.3.1. Three dimensional beam elements87   |  |  |  |
|         | 2.3.2. Two dimensional solid elasticity elements88                                     |  |  |  |
|         | 2.3.3. Boundary elements   |  |  |  |
| 2.4.    | Data check95   |  |  |  |
| 2.5.    | Shear walls computational models98   |  |  |  |
|         | 2.5.1. Symmetrical shear wall models101  |  |  |  |
|         | 2.5.2. Unsymmetrical shear wall models120  |  |  |  |
|         | 2.5.3. Elements describing the shear wall models123                                    |  |  |  |
| 2.6.    | Compatibility between elements124  |  |  |  |
|         |  |  |  |  |
| Chapter | 3: Analysis of Symmetrical Shear Walls125  |  |  |  |
| 3.1.    | Introduction125  |  |  |  |
| 3.2.    | Results of Analysis125   |  |  |  |
|         | 3.2.1. Effect of shear wall breadth126   |  |  |  |
|         | 3.2.1.1. Lateral deflections of the wall126  |  |  |  |
|         |  |  |  |  |
|         | 3.2.1.2. Stresses under wall foundation128   |  |  |  |
|         | 3.2.1.2. Stresses under wall foundation128 3.2.1.3. Internal Forces in Lintel Beams130 |  |  |  |
|         |  |  |  |  |
|         | 3.2.1.3. Internal Forces in Lintel Beams130  |  |  |  |
|         | 3.2.1.3. Internal Forces in Lintel Beams130 3.2.2. Effect of opening breadth           |  |  |  |

| 3.2.3. Effect of modulus of elasticity of soil155 |
|---|
| 3.2.3.1. Lateral deflections of the wall155       |
| 3.2.3.2. Stresses under wall foundation158        |
| 3.2.3.3. Internal forces in lintel beams161       |
| 3.2.4. Effect of curtailment165                   |
| 3.2.4.1. Lateral deflections of the wall165       |
| 3.2.4.2. Stresses under wall foundation165        |
| 3.2.4.3. Internal stresses in lintel beams166     |
| 3.2.5. Effect of raft foundation thickness172     |
| 3.2.5.1. Lateral deflections of the wall172       |
| 3.2.5.2. Stresses under wall foundation172        |
| 3.2.5.3. Internal forces in lintel beams172       |
|   |
| Chapter 4: Analysis of Unsymmetrical Shear Walls  |
| 4.1. General                                      |
| 4.2. Results of the Analysis178                   |
| 4.2.1. Internal Forces in Lintel Beams            |
| 4.2.1.1. Bending Moments                          |
| 4.2.1.2. Shearing Forces                          |
| 4.2.1.3. Normal Forces186                         |
| 4.2.2. Stresses Under Wall Foundation187          |
| 4.2.3. lateral deflections of Walls194            |
| 4.2.4. Stresses in the Shear Walls at different   |
| levels 196  |

| Chapter       | 5: Conc    | lusions and Recomendations217           |  |  |
|---------------|------------|---|--|--|
| 5.1.          | General217 |   |  |  |
| 5.2.          | Symmetr    | ical Shear Walls217                     |  |  |
|               |            |   |  |  |
|               | 5.2.1.     | Effect of the variation of shear wall   |  |  |
|               |            | breadth(B)21                            |  |  |
|               | 5.2.2.     | Effect of the variation of the opening  |  |  |
|               |            | breadth219                              |  |  |
|               | 5.2.3.     | Effect of the variation of the modulus  |  |  |
|               |            | of elasticity of soil { E soil ]221     |  |  |
|               | 5.2.4.     | Effect of the curtailment at the ground |  |  |
|               |            | level221                                |  |  |
|               | 5.2.5.     | Effect of the variation of the raft     |  |  |
|               |            | foundation thickness222                 |  |  |
| 5.3.          | Unsymmet   | trical Shear Walls                      |  |  |
|               | 5.3.1.     | Effect of the variation of the          |  |  |
|               |            | breadth of the openings (b)223          |  |  |
| 5.4.          | Recomme    | ndations225                             |  |  |
|               |            |   |  |  |
| REFERENC      | ES         |   |  |  |
|               |            |   |  |  |
| Appendix AA.1 |            |   |  |  |
| Innandiy D    |            |   |  |  |



#### INTRODUCTION

The accumulation of residential and business establishments at the central zones of main cities, together with the increasing cost of land, requires the vertical extension of the constructed buildings for social and economical reasons.

Such high rise buildings may be defined as those in which the effect of the lateral forces, such as wind and earthquake, are more dominant than the gravity forces, thus necessitating careful study of the design and stability of the structures.

Since the late nineteenth century when the first skyscrapers were built, the main aim of structural engineers and architects has been to achieve efficient and economical structural systems for various ranges and heights of buildings going all the way up as well as 100 stories.

Advanced researches on building materials such as reinforced concrete and structural steel allowed for an efficient use of these materials in choosing an economical method of building tall structures.

The process of choosing the structural system for tall buildings depends on many aspects, one of them may include the foundation considerations. For example, tall buildings having rock soil available near the ground level, is considered to have stable foundation, for this case the structural system or materials are not affected. The structural system also depends upon the overall proportions of the tower, the height to width ratio accompanied with the floor area ratio of the building. Climatic conditions including the wind effects and the seismic zone in which the structure lies, is of utmost importance. The structural system considering the floors deflections, wind, earthquake sway must be in coordination with the architectural, structural and mechanical system requirements.

The available choices of reinforced concrete structural systems are:

- a) traditional beam column framing, which may or may not be simulated by flat slabs or waffle constructions, this system can be suitable up to 20 stories.
- b) shear wall systems in the direction of lateral forces and continuous shear walls up to 150 stories
- c) shear wall frame interaction where shear walls interact with beam-column or slab-column type of framing, suitable for up to 70 stories.
- d) framed-tube created by closely spaced exterior columns

tied at each floor with deep spandrel beams simulating a hollow tube with perforated holes (window openings) suitable up to 60 stories

- e) tube in tube system where an exterior closely spaced column and spandrel beams creates an exterior tube interacting with an interior tube made by shear walls enclosing the surface core, suitable up to 100 stories.
- f) modular tube system where an exterior closely spaced column spandreled tube is stiffened by walls or grid walls dividing the tube into two or more smaller tubes within the boundary of the overall building.

Another available choice of structural systems is the structural steel systems (frames and trusses) or a composite structural system for example a concrete exterior framed tube accompanied with a steel interior framing.

The structural system studied in this thesis is the reinforced concrete system shear walls. In general shear walls are considered so rigid in their own plane so as to resist almost all the lateral loads imposed on the structure. They are well suited for constructions in earthquake areas and one of its main advantages is the low reinforcing steel content.