Ain Shams University Faculty of Engineering Structural Engineering Department



Ahmed Hosny Mohamed Abdel-Rahman

B. Sc. Civil Eng. - Hon. (1989) Ain Shams University

A Thesis

Submitted in Partial Fulfillment for the Requirements of the Degree of Master of Science in Civil Engineering

Supervised By

A.H

Prof. Dr. Fathalla Mohamed El-Nahhas
Prof. of Soil Mechanics and Foundation Engineering
Ain Shams University

Dr. Ali Abdel-Fattah Ali Assist. Prof. of Soil Mech. and Found. Ain Shams University Dr. Mohamed Adel El-Gammal Researcher in Eng. Research Inst. National Research Center

1 -

Cairo - 1993

بسم الله الرحمن الرحيم



EXAMINER COMMITTEE

Name, Title, Affiliation

Signature

- 1. Prof. Dr. Farouk I. El-Kadi
 Prof. of Geotech. Engineering
 Ain Shams University
- 2. Prof. Dr. Moustafa K. El-Ghamrawy
 Prof. of Geotech. Engineering
 El-Azhar University
- 3. Prof. Dr. Fathalla M. El-Nahhas
 Prof. of Geotech. Engineering
 Ain Shams University

Date: / /1993

STATEMENT

This dissertation is submitted to Ain Shams University for the degree of M. Sc. in Civil Engineering.

This work included in the thesis was carried out by the author in the department of Structural Engineering, Ain Shams University, from April, 1992 to June 1993.

No part of this thesis has been submitted for a degree or a qualification at any other university or institution.

| Date | : | - | 1993 | | |
|-----------|--------|-------|---------------|-------------|----|
| Signature | e: | | • • • • • • • | | |
| Name | :Ahmed | Hosny | Mohamed | Abdel-Rahma | ır |

ACKNOWLEDGMENTS

I wish to express my thanks and appreciation to Prof. Dr. Fathalla El-Nahhas for his supervision, guidance and both technical and moral support through this research. I also wish to express my deep gratitude to Dr. Ali Abdel-Fatah for his encouragement, invaluable advices and guidance during each stage of this research. Many thanks go to Dr. Adel El-Gammal for his continuous helpful discussions for every part of this thesis.

My utmost gratitude is for Dr. Mohamed Sheta for his encouragement, enormous help and sincere advices that gave me the strength to finish this work. Special thanks should go to Dr. Ashraf Abdel-hy for his friendship and helpfull technical support. Finally, I wish to dedicate this work to my family for giving me the suitable atmosphere during the time spent on this study.

Ain Shams University
Faculty of Engineering
Structural Engineering Department

Abstract for the M. Sc. thesis submitted by Eng. Ahmed Hosny Mohamed Abdel-Rahman

Title of the Thesis: Numerical Modelling of Concrete Diaphragm Walls

Supervisors : 1. Prof. Dr. Fathalla Mohamed El-Nahhas

2. Dr. Ali Abdel-Fattah Ali

3. Dr. Mohamed Adel Ghareb El-Gammal

Registration Date : 10/12/1990 Examination Date : 4/9/1993

ABSTRACT :

Implementation of subway systems became one of the effective solutions to overcome some of the environmental problems which face man's life sepecially in large highly populated cities.

Construction of subway stations and tunnels using the cut-and-cover technique employing reinforced concrete diaphragm walls is considered one of the most effective methods for controlling ground movements with minor effects on the adjacent existing structures. Analysis of such braced walls is considered one of the most sophisticated soil-structure interaction problems.

This research proposes a numerical modelling for analysis of diaphragm walls during the different construction stages of tunnels and subway stations. The associated variations in the soil stress field and deformations were investigated. In order to carry out such analysis, a computer program was specially upgraded using the Fortran language. Eight-node isoparametric finite elements were used to simulate the soil continuum and the diaphragm wall, the nonlinear stress-strain behaviour of the soil employing modified Duncan model (1984) were utilized. Spring type interface elements were also introduced to model the soil-diaphragm wall contact surface.

This thesis presents a comparative study between results of the nonlinear finite element analysis and the field measurements which had been compiled during construction of the Greater Cairo Underground Metro, Phase 1. Also, results of the analysis were compared with the predicted values from the commonly used empirical design rules of such walls.

An extensive parametric study was also carried out to evaluate the sensitivity of the utilized numerical model and the effect of different factors concerning the soil-wall interaction on the behaviour of the diaphragm wall and the associated soil deformations and stress field.

Key words: Diaphragm walls, tunnels, braced excavation, nonlinear analysis, Fortran, Finite element, eight node element, interface elements, unloading increments.

TABLE OF CONTENTS

| СН | APTER PAGE |
|----|---|
| 1. | INTRODUCTION |
| | 1.1 GENERAL |
| | 1.2 NATURE OF THE PROBLEM2 |
| | 1.3 OBJECTIVE OF THE RESEARCH3 |
| | |
| 2. | LITERATURE SURVEY |
| | 2.1 INTRODUCTION4 |
| | 2.2 CONSTRUCTION OF BRACED EXCAVATION4 |
| | 2.3 DIFFERENT METHODS OF ANALYSIS6 |
| | 2.3.1 METHODS BASED ON THE EARTH PRESSURE |
| | THEORIES6 |
| | 2.3.1.1 THE WEDGE THEORY7 |
| | 2.3.1.2 RANKINE EARTH PRESSURE7 |
| | 2.3.1.3 SOIL PRESSURE ON BRACED SHEETING9 |
| | 2.3.2 SUBGRADE REACTION METHOD13 |
| | 2.3.3 FINITE ELEMENT ANALYSIS METHOD |
| | 2.4 METHODS FOR THE PREDICTION OF THE GROUND LOSS |
| | AROUND THE EXCAVATION20 |
| | 2.4.1 MATHEMATICAL PRESENTATION FOR THE |
| | SETTLEMENT DISTRIBUTION20 |
| | 2.4.2 METHODS BASED ON THE DATA BASE RESULTS21 |
| | 2.5 MISCELLANEOUS ASPECTS AND DIFFERENT CASE |
| | HISTORIES23 |
| | 2.5.1 BASIC MOVEMENT TREND25 |
| | 2.5.1.1 MAXIMUM MOVEMENTS - STIFF CLAYS, |
| | RESIDUAL SOILS AND SANDS25 |

| 2.5.1.2 | MAXIMUM MOVEMENTS - SOFT AND MEDIUM |
|-----------------------|---------------------------------------|
| | CLAYS31 |
| 2.5.1.3 | GENERAL PATTERN OF GROUND |
| | MOVEMENTS33 |
| 2.5.1.4 | SETTLEMENT PROFILES ADJACENT TO |
| | EXCAVATION35 |
| 2.5.1.5 | EFFECT OF THE DIAPHRAGM WALL |
| | INSTALLATION METHOD ON THE SETTLEMENT |
| | ARROUND THE EXCAVATION35 |
| 2.5.2 ACTUAL | LATERAL STRESS DISTRIBUTION BEHIND |
| THE BRA | CED EXCAVATION38 |
| 2.5.2.1 | DIFFICULTIES ASSOCIATED WITH THE USE |
| | OF PECK'S EARTH PRESSURE |
| | DISTRIBUTION, (1969)38 |
| 2.5.2.2 | PROPOSED APPROACH FOR ESTIMATING THE |
| | ACTUAL EARTH PRESSURE |
| | DISTRIBUTION39 |
| | |
| 3. METHOD OF ANALYSIS | |
| 3.1 INTRODUCTION. | 41 |
| 3.2 GENERAL DESCR | IPTION OF THE ANALYSIS42 |
| 3.3 IDEALIZATION | OF THE PROBLEM45 |
| 3.3.1 THE EIG | HT-NODE ISOPARAMETRIC ELEMENT |
| STIFFNE | SS MATRIX46 |
| 3.3.1.1 | THE EIGHT NODES ELEMENT |
| | CONFIGURATION46 |
| 3.3.1.2 | EIGHT NODES ELEMENT STRAIN AND |
| | DISPLACEMENT49 |
| | |

| | | | 3.3.1.3 | THE CONSTITUTIVE MATRIX52 |
|----|------|--------|----------|--------------------------------------|
| | | | 3.3.1.4 | THE EIGHT NODES ELEMENT STIFFNESS |
| | | | | MATRIX FORMULATION53 |
| | | 3.3.2 | CONSTITU | UTIVE MODEL55 |
| | | | 3.3.2.1 | NONLINEAR SOIL MODEL55 |
| | | | 3.3.2.2 | HISTORY AND DESCRIPTION OF THE |
| | | | | HYPERBOLIC STRESS-STRAIN MODEL56 |
| | | | 3.3.2.3 | NONLINEAR INCREMENTAL FINITE ELEMENT |
| | | | | METHODOLOGY59 |
| | | | 3.3.2.4 | THE ORIGINAL DUNCAN MODEL, 198060 |
| | | | 3.3.2.5 | THE MODIFIED HYPERBOLIC MODEL, |
| | | | | DUNCAN, 198463 |
| | | | 3.3.2.6 | ANALYSIS PROCEDURES EMPLOYED IN THE |
| | | | | PROGRAM68 |
| | | 3.3.3 | INTERFAC | CE FINITE ELEMENT71 |
| | | | 3.3.3.1 | INTERFACE ELEMENT STIFFNESS |
| | | | | MATRIX71 |
| | | | 3.3.3.2 | INTERFACE ELEMENT MODEL |
| | 3.4 | NUMERI | CAL SIM | JLATION FOR BRACED EXCAVATION77 |
| | | 3.4.1 | SOIL EXC | CAVATION77 |
| | | 3.4.2 | INSTALLA | ATION OF TEMPORARY STRUTS79 |
| | | 3.4.3 | DELETION | N OF TEMPORARY STRUTS80 |
| | | | | |
| 4. | PARA | METRIC | STUDY A | AND CASE HISTORY |
| | 4.1 | INTRO | OUCTION | 81 |
| | 4.2 | FINITE | ELEMENT | r ANALYSIS FOR A CASE HISTORY84 |
| | | 4.2.1 | CONSTRUC | CTION PROCEDURE FOR THE RUNNING |
| | | | TUNNEL | |

| | 4.2.2 | MATERIAL PROPERTIES AND PARAMETERS USED IN |
|-----|--------|---|
| | | THE ANALYSIS87 |
| | 4.2.3 | FINITE ELEMENT MESH CONFIGURATION90 |
| | 4.2.4 | PRESENTATION OF THE FINITE ELEMENT |
| | | RESULTS90 |
| | 4.2.5 | FINITE ELEMENT RESULTS OF THE CASE |
| | | HISTORY90 |
| | 4.2.6 | COMPARISON BETWEEN THE FIELD MEASUREMENTS |
| | | AND THE RESULTS OF THE FINITE ELEMENT |
| | | ANALYSIS99 |
| | | 4.2.6.1 HORIZONTAL DEFORMATION OF THE |
| | | WALL100 |
| | | 4.2.6.2 SETTLEMENT TROUGH ADJACENT TO THE |
| | | WALL AND HEAVE INSIDE THE |
| | | EXCAVATION100 |
| | 4.2.7 | COMPARISON BETWEEN THE COMMON DESIGN RULES OF |
| | | THE BRACED WALLS AND THE RESULTS OF THE |
| | | FINITE ELEMENT ANALYSIS104 |
| 4.3 | PARAMI | ETRIC STUDY108 |
| | 4.3.1 | EFFECT OF THE CONSTITUTIVE MODEL OF SOIL108 |
| | 4.3.2 | EFFECT OF NONLINEAR PARAMETERS114 |
| | | 4.3.2.1 EFFECT OF MODULUS NUMBER (K)115 |
| | | 4.3.2.2 EFFECT OF BULK MODULUS NUMBER |
| | | (Kb)125 |
| | 4.3.3 | EFFECT OF VARIATION OF THE WALL |
| | | STIFFNESS134 |
| | 4.3.4 | EFFECT OF THE PRESENCE OF THE BRACING |
| | | SYSTEM140 |

| | 4.3.5 EFFECT OF VARIATION OF THE STRUT | |
|-----|--|-----|
| | STIFFNESS | 146 |
| | 4.3.6 EFFECT OF VARYING THE NUMBER OF THE LOAD | |
| | INCREMENTS USED IN THE ANALYSIS | 147 |
| 4.4 | PRECONSTRUCTION PREDICTION OF THE BEHAVIOUR OF | |
| | UNDERGROUND STATIONS OF LINE 2 OF CAIRO METRO | 153 |
| | 4.4.1 STATION DIMENSIONS AND THE SOIL MODEL | 153 |
| | 4.4.2 CONSTRUCTION SEQUENCES FOR THE STATION | 159 |
| | 4.4.3 RESULTS OF THE FINITE ELEMENT ANALYSIS | 161 |
| | | |
| | | |
| 5. | UMMARY, CONCLUSIONS AND RECOMMENDATIONS FOR FURTHE | R |
| | TUDIES | |
| | .1 INTRODUCTION | 165 |
| | .2 SUMMARY | 165 |
| | .3 CONCLUSIONS | 167 |
| | .4 RECOMMENDATIONS FOR FURTHER STUDIES | 172 |
| | | |
| | reforncec | 171 |

LIST OF FIGURES

| FIG. | . NO. P | AGE |
|------|---|-----|
| | | |
| 2.1 | Wedge equilibrium | 8 |
| 2.2 | Illustration of active and passive pressures for | |
| | cohesionless and cohesive soil. (After Bowels, | |
| | 1982) | 10 |
| 2.3 | Development of earth pressure on braced excavations | 12 |
| 2.4 | Lateral earth pressure diagrams as suggested by Pec | k, |
| | 1969 | 14 |
| 2.5 | Lateral earth pressure diagrams against braced | |
| | sheeting as proposed by Tshebotarioff (1973) | 15 |
| 2.6 | Determination of the modulus of subgrade reaction | |
| | from insitu test results | 16 |
| 2.7 | Schematic diagram illustrates Caspe (1969) | |
| | analytical method for estimating the sett. | |
| | distribution adjacent to the excavation | 22 |
| 2.8 | Summary of soil settlements behind insitu walls | |
| | (After Peck, 1969) | 24 |
| 2.9 | Maximum lateral movement for the insitu walls and | |
| | settlements in the soil retained by insitu walls in | |
| | stiff clays, residual soils and sands (After Clough | |
| | and O'Rourke, 1990 | 27 |
| 2.10 | Summary of measured settlements and horizontal | |
| | displacement adjacent to excavations in stiff to | |
| | very hard clay (After Clough and O'Rourke, 1990) | 28 |
| 2 11 | Predicted maximum lateral wall movement by finite | |

| | element analysis modelling stiff soil conditions | |
|------|--|----|
| | (After Clough and O'Rourke, 1990) | 30 |
| 2.12 | 2 Design curves to obtain maximum lateral wall | |
| | movement and settlement for soft to medium clays | |
| | (Afetr Clough and O'Rourke, 1990) | 32 |
| 2.13 | Typical profiles of movement for braced and tied | |
| | back walls (After Clough and O'Rourke, 1990) | 34 |
| 2.14 | Dimensionless settlement profiles recommended for | |
| | estimating the distribution of settlement adjacent | |
| | to excavations in different soil types (After Clough | |
| | and O'Rourke, 1990) | 36 |
| 2.15 | Summary of measured settlements caused by the | |
| | installation of concrete diaphragm walls (After | |
| | Clough and O'Rourke, 1990) | 37 |
| | | |
| 3.1 | The program flow chart | 43 |
| 3.2 | Local coordinates system for the eight nodes | |
| | element | 47 |
| 3.3 | Hyperbolic model for stress-strain behaviour | |
| | (After Duncan et al., 1984) | 61 |
| 3.4 | Modelling unloading-reloading moduli without | |
| | introducing computational instability (After | |
| | Duncan et al, 1984 | 66 |
| 3.5 | Spring type interface element | 72 |
| 3.6 | Stress-deformation curves for the interface | |
| | elements (After Dessouki, 1985) | 75 |
| 3.7 | Hyperbolic simulation of the experemental direct | |
| | | |

| 3.8 | Simulation of the excavation process in the F.E. | |
|------|--|-----|
| | analysis | 78 |
| | | |
| 4.1 | Cross-section details of the tunnel section and | |
| | the soil profile at this location (After El-Nahhas | |
| | et al, 1988) | 82 |
| 4.2 | Genearal layout of instrumentation at the tunnel | |
| | test section (After Shalaby, 1989) | 83 |
| 4.3 | Construction procedures steps for the running | |
| | tunnel test section (After El-Nahhas et al., 1988) | 85 |
| 4.4 | Simulation of the construction stages in the finite | |
| | element analysis | 86 |
| 4.5 | The finite element mesh used in the analysis of | |
| | stages 3 to 7 | 91 |
| 4.6 | The finite element mesh used in the analysis of | |
| | stages 1 and 2 | 92 |
| 4.7 | Propagation of the computed horizontal movement | |
| | during the different construction stages | 94 |
| 4.8 | Propagation of the computed settlement adjacent to | |
| | the wall during the different construction stages. | 95 |
| 4.9 | Propagation of the computed heave inside the | |
| | excavation during the different construction stages. | 96 |
| 4.10 | Distribution of the lateral earth pressure on the | |
| | diaphragm wall during the different construction | |
| | stages | 97 |
| 4.11 | Comparison between the results of the finite element | |
| | analysis and the field measurements for the | |
| | horizontal deformation at stage No 6 | 101 |