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# SIOLOGICAL \_ EXTENDED PHYSICO \_ CHEMICAL

TREATMENT OF WASTEWATER

Ph.D.Thesis Presented to the
Faculty of Science
Ain Shams University
For The Degree of Doctor of Philosophy
In Science

BY

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1986

# BIOLOGICAL-EXTENDED PHYSICO-CHEMICAL TREATMENT OF WASTEWATER

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#### ACKNOWLEDGEMENT

I am greatly indepted to Prof. A.M. Samour, Dean of Faculty of Science, Ain Shams University, for sponsoring this research work.

I wish to express my deep sense of gratitude to Professor Fatma A. El-Gohary, Head of the Water Pollution Control Laboratory, National Research Centre, for the helpful supervision, her masterly guidance, suggesting the subject of this thesis, and for discussing the results.

I am also grateful to Prof. M. Anwar El-Dib, Water Pollution Control Laboratory, National Research Centre, for his helpful, keen interest and encouragement.

Thanks are also due to professor Dr. Eng. U. Neis, and Dipl.-Eng.P. Pinnow, Karlsruhe University, Federal Republic of Germany, for designing the pilot plant and their great help and cooperation during the progress of this investigation.

Thanks are due to Associated professor S. El-Hawary, Water Pollution Control Laboratory; National Res. Centre, for his valuable help.

I should not forget to express my sincere thanks to all members of Water Pollution Control Laboratory for their kind cooperation and friendship.

### ABBREVIATIONS AND SYMBOLS

BOD Biochemical oxygen demand.

BOD (sol.) Dissolved biochemical oxygen demand.

BOD<sub>T</sub> Total biochemical oxygen demand.

COD Chemical oxygen demand.

COD (sol.) Dissolved chemical oxygen demand.

ECD Electron capture detector.

FID Flame ionization detector.

m g d Million gallon per day.

mg/l Milligram per litre.

NSBE Non-settled biological effluent.

P Phosphorus.

RBC Rotating biological contactor.

Res. Residual.

r.p.m. Rotation per minute.

R (%) Removal percentage.

SBE Settled biological effluent.

SS Suspenced solids.

S.V.I Sludge volume index.

TR Total residue.

VOM Volatile organic matter.

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Summary

#### SUMMARY

It is now becoming evident that recycling and reuse of water should be given more consideration specially in arid and semi-arid areas. We can no longer afford the indiscriminate waste of our natural resources, neither should we accept the uncontrolled disposal of wastes.

Experience has, however, shown that health requirements must be given very careful attention from the initial planning stages to ensure that adequate health criteria are established to meet various water use objectives.

Reuse of municipal wastewater for agricultural irrigation is one of the oldest forms of water reclamation. In order to reduce the health risks associated with wastewater reuse in agriculture, the following three strategies are recommended:

The first is by various methods of treatment of waste-water before agricultural use in order to remove or reduce the number of pathogenic micro-organisms. The second involves various irrigation techniques to reduce or prevent direct contact between the crops and the wastewater. The third involves regulation of the type of crops or other forms of agricultural use; so that the risk of disease transmission is minimized.

The main goal of this study was limited to the first strategy. The overall objective was therefore, to produce,

based on local environmental conditions, engineering design data for domestic wastewater treatment.

It was decided to choose a biological treatment system with low power consumption. Low maintenance & operating skill requirements and ability to give the desired degree of treatment for this purpose a rotating biological contactor was selected to achieve the following specific objectives:

- To define the effect of the operational variables that could influence treatment efficiency. The understanding of the effects could contribute to the improved design and operation os such treatment systems.
- To determine the interactions between the variables.
- To formulate the relationship among the treatment efficiency and organic loading.

Being aware that biological treatment is not fully capable of removing harmful chemicals or microorganisms, the biological contactor was followed by chemical coagulation and/or chlorine disinfection.

Experimental investigations have been carried out on a pilot scale plant. It consists of primary, secondary and tertiary treatment. Assembling and operation of the pilot plant took place at a site of the National Research Centre (Dokki, Cairo) within a residential area where a connection to the sewerage system was available.

The primary treatment consists of:

- 1- Coars screen (20 mm bar width)
- 2- Fine screen (1.5 mm openings)
- 3- Pump sump.
- 4- Sedimentation tank (circular)
  - (i) Diameter 1.6m
  - (ii) Depth 1.0 m

The secondary treatment was a four stage Rotating Biological Contactor. Each stage consists of a 2-m diameter 25-discs which are fabricated from expanded polystyrene beeds. The total surface area of the entire unit was  $628 \text{ m}^2$ . The disks were rotated at 2 r.p.m. with approximately 45% of the surface area submerged in the wastewater. The total liquid volume of the system was  $9.62 \text{ m}^3$ .

The chemical treatment consists of one coagulation-flocculation unit and one chlorination reactor to be operated either separately or in series. The coagulants examined were ferric chloride and calcium oxide.

The chemical treatment units were designed for a 0.25  $\text{m}^3/\text{h}$  effluent to be chosen from one of the four cascades. The system was operated from November 1982 to Nov. 1985. During these three years, several series of experiments were conducted. The influent to the system and the effluent from each stage were monitored by collecting 24-hour composite samples at 20-minutes intervals during the period of steady-state operation.

To determine optimum operating conditions required to produce an effluent of the requisite standards for final disposal or reuse daily as well as seasonal variations in physico-chemical characteristics of sewage in the project area were determined.

Available data showed a wide variation in sewage characteristics.  $BOD_5$  ranged from 224 to 855 with an average value of 502 mg  $O_2/1$ . Corresponding COD values were 409, 2338 & 1108, respectively.

In general sewage strength is above normal. A situation which may be attributed to low water consumption.

To assess the impact of the organic load applied to the system on the quality of the treated effluent, two organic loads were examined. The first ranged from 16.8 to 46.7 and the second ranged from 1.1 to 14:4 g. BOD/m<sup>2</sup>/d.

The data obtained from the combined treatment of settled sewage may be summarized as follows:

1- Increasing the organic load applied to the system within the examined range exerted a slight effect on carbonaceous matter elimination BOD & COD removal values up to 94.9 & 95 percent were recorded. On the other hand, organic nitrogen removal was only around 48% and no nitrification took place.

- 2- To achieve high nitrification levels, ammonia load should not exceed 1.4 g  $\rm NH_3-N/m^2/d$ . Also, high detention time is required.
- 3- Effective removal of the organic compounds present in the RBC effluent could be achieved when lime or ferric chloride were used at their optimum dose and without chaving the pH of the effluent. In addition the coagulation-flucculation process achieved significant removal of the phosphorus responsible for eutrophication in case of disposal by dilution.
- 4- Chemical treatment was slightly affected by increasing the organic load applied to the RBC.
- 5- Chlorination of RBC effluent brought about considerable reduction in viable bacterial counts pathogenic microorganisms. The results obtained showed that at constant chlorine dose of 15 mg/l, reduction in tested parameters is proportional to the detention time.

Percentage reductions in total bacterial counts, at 22°C and 37°C, total coliform counts and yeast counts were 99.6, 99.1, 99.8 & 96, respectively.

6- Chlorination resulted in the appearance of new chlorinated products & decrease in the concentration of some organic compounds.

Introduction and Literature Review