



**AIN SHAMS UNIVERSITY
FACULTY OF ENGINEERING
STRUCTURAL ENGINEERING DEPARTMENT**

SHEAR BEHAVIOR OF SELF COMPACTING HIGH STRENGTH CONCRETE DEEP BEAMS

**BY
AHMED MOHSEN ABDEL FATTAH**

B.SC. 2006 Structural Division
Civil Engineering Department
Ain Shams University

A Thesis
Submitted in partial fulfillment for the requirements of the
Degree of Master of Science in Structural Engineering

Supervised by
Prof. Dr. Omar Ali Mousa EL-Nawawy
Emeritus Prof. of Concrete Structures
Faculty of Engineering
Ain Shams University

Prof. Dr. Ahmed Hassan Ghallab
Prof. of Concrete Structures
Faculty of Engineering
Ain Shams University

Faculty of Engineering
Ain Shams University
Cairo-2014

STATEMENT

This thesis is submitted to Ain Shams University, Cairo, Egypt, for the degree of Master of Science in Civil Engineering (Structural).

The work included in this thesis was carried out by the author in the Department of Civil Engineering (Structural Division), Ain Shams University, from August 2012 to April 2013.

No part of this thesis has been submitted for a degree or qualification at any other University or Institute.

Date : / / 2014

Name : Ahmed Mohsen Abdel Fattah

Signature:

**AIN SHAMS UNIVERSITY
FACULTY OF ENGINEERING**

APPROVAL SHEET

Thesis : Master of Science in Civil Engineering (Structural)
Student Name : Ahmed Mohsen Abdel Fattah
Thesis Title : SHEAR BEHAVIOR OF SELF COMPACTING
HIGH STRENGTH CONCRETE DEEP BEAMS

Examiners Committee:

Signature

Prof. Dr. Adel Glal Tawfik Elattar

Professor of R.C. Structures
Faculty of Engineering - Cairo University

.....

Prof. Dr. Ayman Hussein Hosny

Professor of R.C. Structures
Ain Shams University

.....

Prof. Dr. Omar Ali Mousa El Nawawy

Emeritus Professor of R.C. Structures
Ain Shams University
(Supervisor)

.....

Prof. Dr. Ahmed Hassan Ghallab

Professor of R.C. Structures
Ain Shams University
(Supervisor)

.....

Date: / / 2014

INFORMATION ABOUT THE RESEARCHER

Name: Ahmed Mohsen Abdel Fattah

Date of Birth: December 12th, 1983

Place of Birth: Cairo, Egypt

Qualifications: B.Sc. Degree in civil Engineering (Structural Engineering)
Faculty of Engineering, Ain Shams University (2006)

Present Job: Structural Engineer, ECG Engineering Consultants Group

Signature:

ACKNOWLEDGMENTS

I would like to express my sincerest appreciation to my advisors, Prof. Dr. Omar El-Nawawy and Prof. Dr. Ibrahim Shaaban for their guidance, continuous, valuable guidance, and the investments, giving me the opportunity to be involved in such interesting research.

I would like to extend sincere thanks to my advisor, Prof. Dr. Ahmed Ghalab for providing the guidance necessary to complete this research and also for his constant encouragement, support, and friendship which was the motivating force that kept work on my thesis in force until completion. I would like to express my admiration and thanks for his loyalty and trustfulness

Ahmed Mohsen

Eng. / Ahmed Mohsen Abdel Fattah

Title of the thesis:

**SHEAR BEHAVIOR OF SELF COMPACTING HIGH STRENGTH
CONCRETE DEEP BEAMS**

The prime objective of this research is to improve the understanding of the behavior of self compacting high strength concrete deep beams failing in shear, and to study the size effect in self compacting high strength concrete and to evaluate the effect of orthogonal reinforcement on the shear capacity of deep beams and on size effect. Furthermore, we studied the various proposed procedures for predicting the shear strength of deep beams.

In order to achieve these general aims, the following specific objectives are proposed:

- To contribute to the understanding of the mechanism of shear strength in reinforced concrete deep beams with and without shear reinforcement and how the use of self compacting concrete may affect them.
- Experimental program is suggested to study different parameters studied in the thesis and to show their effect on the shear strength of the studied beams.
- Verification of shear strength results using empirical equations from literature.

The thesis is divided into seven chapters. In Chapter 2, a state of the art is presented, in which different conceptual models and shear design procedures for evaluating the failure shear strength of reinforced concrete deep beams with and without web reinforcement are introduced. Moreover, the basic characteristics of self compacting high strength concrete are pointed out and previous experimental research is summarized.

In order to better understand the response of self compacting high strength concrete deep beams that fail in shear, twelve deep beam specimens were designed to ensure shear failure. In Chapter 3 the details of beam specimens, material properties, instrumentation and the testing procedure used are carefully described.

The results obtained from the experimental campaign carried out at the Concrete Research Laboratory at Ain-Shams University are presented and discussed in Chapter 4. The influence of each design parameter is studied separately, and test results are compared with different shear approaches. These results will be discussed in full details in chapter 5.

In Chapter 6, shear design methods based on the observed behavior are reviewed for self compacting high strength concrete deep beams with and without web reinforcement, attempting, however, to point out differences and to keep it simple enough to make it suitable for implementation in a code of practice.

Finally, Chapter 7 presents general and specific conclusions, together with recommendations for future research.

The most important conclusion were :

- Self compacting high strength concrete deep beams exhibited size effect which is reduction in the ultimate shear stresses with the increase of the height of the deep beam, in the same manner as high strength concrete and ordinary strength concrete that's why such behaviour should be considered in the design equations.
- The orthogonal web reinforcement enhanced the ultimate shear strength of the tested deep beams, but it did not remove the size effect, the ultimate shear capacity of the section decreased with the increase of the depth in the same manner exhibited by specimens with no orthogonal web reinforcement when the height varied from 500 mm to 900mm.
- On comparing the effect of the vertical web reinforcement and the horizontal web reinforcement on size effect, it was found that the latter had greater effect on decreasing the impact of increasing the depth on the ultimate shear stress of the tested deep beams.

CONTENTS

ACKNOWLEDGMENTS	v
ABSTRACT	vi
CONTENTS	viii
LIST OF FIGURES	xii
LIST OF TABLES	xviii
CHAPTER 1: INTRODUCTION	
1.1 Background	1
1.2 Purpose of Investigation	2
1.3 Structure of the Thesis	3
CHAPTER 2: LITERATURE REVIEW	
2.1 Introduction	7
2.2 High Strength Concrete	8
2.2.1 Design of High Strength Concrete Mixture	8
2.2.2 Potential applications of high strength concrete.....	9
2.3 Self Compacting Concrete.....	10
2.3.1 Development of self-compacting concrete.....	11
2.3.2 Mechanism for achieving self-compactability	12
2.3.3 Why is SCC used.....	16
2.3.4 SCC Engineering properties	16
2.3.5 Comparison between SCC & OC with different levels of pozzolanic reactions)	18
2.4 Behavior of deep beams	21
2.4.1 Basic shear theory in homogeneous beams	21
2.4.2 Truss model for beams failing in shear	24
2.4.3 Compressive force path method	26
2.4.4 Behavior of deep beams	28
2.5 Factors affecting deep beams behavior	30
2.5.1 Concrete strength (F _{cu})	30
2.5.2 Span depth ration (l _e /d)	31
2.5.3 Shear span ratio a/d	32
2.5.4 Main tension steel.....	33
2.5.5 Vertical web reinforcement	34
2.5.6 Horizontal web reinforcement.....	35

2.5.7 Orthogonal web reinforcement.....	36
2.5.8 Loading pattern.....	37
2.6 Size effect in deep beams	38
2.6.1 Introduction:	38
2.6.2 Studied Factors affecting size effect in deep beams:	40
2.6.2.1 Concrete compressive strength:.....	40
2.6.2.2 Shear span a/d	41
2.6.2.3 Beam width.....	42
2.6.2.4 Dimension of support and loading plates	42
2.6.3 Size effect information	43
2.6.4 Current conclusion on size effect in deep beams	44
CHAPTER 3: EXPERIMENTAL WORK	
3.1 Introduction	47
3.2 Materials	47
3.2.1 Coarse aggregate and Sand.....	47
3.2.2 Cement.....	47
3.2.3 Mixing water	47
3.2.4 Steel reinforcement.....	48
3.2.5 Silica Fumes	48
3.2.6 Super plasticizer	48
3.3 Concrete Mix.....	49
3.4 Mixing and preparation of beams.....	50
3.5 Material properties	54
3.6 Specimen Details.....	54
3.7 Instrumentation and Testing Procedure.....	58
CHAPTER 4: EXPERIMENTAL RESULTS	
4.1 Introduction	67
4.2 Cracking Patterns	68
4.2.1 Without web reinforcement (Group 0):.....	70
4.2.2 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=0.5\%$) (Group 1)	74
4.2.3 With web reinforcement ($\rho_{vl}=1.0\%$), ($\rho_{hz}=0.5\%$) (Group 2).....	78
4.2.4 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=1.0\%$) (Group 3).....	82
4.3 Load-deflection relationship:	86
4.4 The Cracking & the Ultimate load:	94
4.4.1 Without web reinforcement (Group 0).....	95

4.4.2 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=0.5\%$) (Group 1).....	95
4.4.3 With web reinforcement ($\rho_{vl}=1.0\%$), ($\rho_{hz}=0.5\%$) (Group 2).....	95
4.4.4 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=1.0\%$) (Group 3).....	95
4.5 Modes of Failure	96
4.5.1 Without web reinforcement (Group 0):.....	97
4.5.2 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=0.5\%$) (Group 1)	99
4.5.3 With web reinforcement ($\rho_{vl}=1\%$), ($\rho_{hz}=0.5\%$) (Group 2)	100
4.5.4 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=1\%$) (Group 3)	102
4.6 Strain measurements.....	104

CHAPTER 5: DISCUSSION OF TEST RESULTS

5.1 Introduction	118
5.2 Discussion of results of the tested beams	119
5.2.1 Effect of depth (d)	119
5.2.1.1 Without web reinforcement (Group 0):.....	119
5.2.1.1.1 Cracking Pattern	119
5.2.1.1.2 Load-Deflection behavior.....	122
5.2.1.1.3 Cracking and ultimate strength.....	123
5.2.1.1.4 Mode of Failure	125
5.2.1.2 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=0.5\%$) (Group 1).....	126
5.2.1.2.1 Cracking Pattern	126
5.2.1.2.2 Load-Deflection behavior.....	129
5.2.1.2.3 Cracking and ultimate strength.....	129
5.2.1.2.4 Modes of failure	130
5.2.1.3 With web reinforcement ($\rho_{vl}=1.0\%$), ($\rho_{hz}=0.5\%$) (Group 2).....	131
5.2.1.3.1 Cracking Pattern	131
5.2.1.3.2 Load-Deflection behavior.....	134
5.2.1.3.3 Cracking and ultimate strength.....	134
5.2.1.3.4 Modes of failure	136
5.2.1.4 With web reinforcement ($\rho_{vl}=0.5\%$), ($\rho_{hz}=1.0\%$) (Group 3).....	136
5.2.1.4.1 Cracking Pattern	137
5.2.1.4.2 Load-Deflection behavior.....	140
5.2.1.4.3 Cracking and ultimate strength.....	140
5.2.1.4.4 Modes of failure	141
5.2.2 Effect of web reinforcement:.....	141
5.2.2.1 Effect of orthogonal web reinforcement (Group 4):	142

5.2.2.1.1 Cracking Pattern	142
5.2.2.1.2 Load-Deflection behavior.....	142
5.2.2.1.3 Cracking and ultimate load.....	144
5.2.2.1.4 Mode of failure	145
5.2.2.2 Effect of vertical & horizontal web reinforcement (Group 5).....	145
5.2.2.2.1 Cracking pattern	146
5.2.2.2.2 Load-Deflection behavior.....	146
5.2.2.2.3 Cracking and ultimate load.....	148
5.2.2.2.4 Mode of failure	149

CHAPTER 6: THEORETICAL ANALYSIS

6.1 Introduction	151
6.2 Methods to calculate the ultimate shear strength of RC deep beams.....	151
6.2.1 Empirical method Egyptian Code of Practice [ECP-2007].....	151
6.2.2 CIRIA Guidelines [1977]	155
6.2.3 Zsutty's formula[1971].....	157
6.2.4 Hsu-Mau's explicit method (Softened truss model)[1987]	157
6.2.5 Strut and tie method	158
6.2.5.1 Basic concepts	159
6.2.5.2 ACI 2011 STM model.....	161
6.2.6 Size effect formulas.....	161
6.3 Comparison between Self compacting concrete, ordinary concrete & high strength concrete.....	165
6.4 Experimental results vs. theoretical results	166

CHAPTER 7: CONCLUSIONS AND RECOMMENDATIONS

7.1 Summary	177
7.2 Conclusions & Recommendations.....	178
7.3 Future Work.....	180

REFERENCES

LIST OF FIGURES

Figure (2-1) SCC with a slump flow of 29-inches (725-mm) tested by the slump flow test.....	10
Figure (2-2) Necessity for Self-Compacting Concrete	11
Figure (2-3) Comparison of mix proportioning between Self-Compacting Concrete and conventional concrete	12
Figure (2-4) Necessity for Self-Compacting Concrete.....	12
Figure (2-5) Mechanism for achieving self-compactability.....	13
Figure (2-6) Comparison of mix-proportioning of SCC with other types of conventional concrete	14
Figure (2-7) Degree of aggregate's compaction Coarse aggregate in concrete and fine aggregate in mortar. (Okamura H. & Ouchi M. 2002)	15
Figure (2-8) Relationship between paste volume and water powder ratio. (Okamura H. & Ouchi M. 2002)	15
Figure (2-9) Compressive strength (Water/Binder=0.40). (Ahmadi M.A., Alidoust O. etal 2007)	19
Figure (2-10) Compressive strength (Water/Binder=0.35). Ahmadi M.A., Alidoust O. etal 2007)	19
Figure (2-11) Modulus of elasticity. (Ahmadi M.A., Alidoust O. etal 2000).....	20
Figure (2-12), Internal Forces in Beam	21
Figure (2-13) Distribution of flexural shear stresses	22
Figure (2-14) Principal Stresses	23
Figure (2-15) Stress Trajectories	24
Figure (2-16) Potential Crack pattern	24
Figure (2-17) Truss analogy for beams failing in shear	25
Figure (2-18) Truss analogy for beams failing in shear	26
Figure (2-19) Compressive force path	27
Figure (2-20) Effect of bond failure on stress conditions in compressive zone	28
Figure (2-21a) Path of compressive force and corresponding outline of compressive stress trajectories for RC beams with various a/d ratios	29
Figure (2-21b) Path of compressive force and corresponding outline of compressive stress trajectories for a typical deep	30
Figure (2-22) Diagonal and ultimate shear stress versus a/d at $\mu_t = 1.56\%$) (Oh J. and Shin S. 2001)	32

Figure (2-23) Diagonal and ultimate shear stress versus a/d at $\rho_t = 1.29\%$ (Oh J. and Shin S. 2001)	32
Figure (2-24) Variation of total ultimate shear strength with ρ_t and a/d (Tan K, Teng S. ,etal 1997)	33
Figure (2-25) Ultimate shear stress versus vertical shear reinforcement ratio ρ_v (Oh J. and Shin S. 2001).....	34
Figure (2-26) Ultimate shear stress versus horizontal shear reinforcement ratio ρ_h (Oh J. and Shin S. 2001)	35
Figure (2-27) Ultimate shear stress versus horizontal shear reinforcement ratio ρ_h (Oh J. and Shin S. 2001)	35
Figure (2-28) ultimate shear stress versus a/d at $\rho_t = 1.56\%$; (Oh J. and Shin S. 2001)	36
Figure (2-29) Ultimate and cracking shear stress for $f_c' = 31.4 \text{ MPa}$ (Yang K,etal. 2003)	40
Figure (2-30) Ultimate and cracking shear stress for $f_c' = 78.5 \text{ MPa}$ (Yang K,etal. 2003)	40
Figure (2-31) Normalized ultimate stress and diagonal cracking stress versus overall height (Tan K.H. and Lu H.Y.1999)	41
Figure (2-32) Histogram of beam depths (number of test data each beam depth of interval 5 in. (130mm) versus depth in inches)	42
Figure (2-33) Example of contamination of database due to variation of uncertain factors other than size, showing some typical test data included in ACI-445f database	43
Figure (3-1) Preparing the tested specimens in wooden forms	51
Figure (3-2) Pouring of concrete in wooden forms	52
Figure (3-3) Pouring of concrete in wooden forms	52
Figure (3-4) Concrete casted in all tested	53
Figure (3-5) Typical elevation & section for tested deep beams	56
Figure (3-6) Section (A-A) in beams B0-500 , B1-500 , B2-500 & B3-500	56
Figure (3-7) Section (A-A) in beams B0-700 , B1-700 , B2-700 & B3-700	57
Figure (3-8) Section (A-A) in beams B0-900 , B1-900 , B2-900 & B3-900	57
Figure (3-9) Test setup for typical specimen	58
Figure (3-10) LVDTs layout for typical tested deep beam	59
Figure (3-11) B0-500 strain gauges layout	59
Figure (3-12) B0-700 strain gauges layout	60

Figure (3-13) B0-900 strain gauges layout	60
Figure (3-14) B1-500 strain gauges layout	61
Figure (3-15) B1-700 strain gauges layout	61
Figure (3-16) B1-900 strain gauges layout	62
Figure (3-17) B2-500 strain gauges layout	62
Figure (3-18) B2-700 strain gauges layout-.....	63
Figure (3-19) B2-900 strain gauges layout	63
Figure (3-20) B3-500 strain gauges layout	64
Figure (3-21) B3-700 strain gauges layout	64
Figure (3-22) B3-900 strain gauges layout	65
Figure (4-1a) Beam B0-500	71
Figure (4-1b) Beam B0-700	71
Figure (4-1c) Beam B0-900	72
Figure (4-1d) Crack width progress for B0-500	72
Figure (4-1e) Crack width progress for B0-700	73
Figure (4-1f) Crack width progress for B0-900	73
Figure (4-2a) Beam B1-500	75
Figure (4-2b) Beam B1-700	75
Figure (4-2c) Beam B1-900	76
Figure (4-2d) Crack width progress for B1-500	76
Figure (4-2e) Crack width progress for B1-700	77
Figure (4-2f) Crack width progress for B1-900	77
Figure (4-3a) Beam B2-500.....	79
Figure (4-3b) Beam B2-700	79
Figure (4-3c) Beam B2-900	80
Figure (4-3d) Crack width progress for B2-500	80
Figure (4-3e) Crack width progress for B2-700	81
Figure (4-3f) Crack width progress for B2-900	81
Figure (4-4a) Beam B3-500	83
Figure (4-4b) Beam B3-700	84
Figure (4-4c) Beam B3-900	84
Figure (4-4d) Crack width progress for B3-500	85
Figure (4-4e) Crack width progress for B3-700	85
Figure (4-4f) Crack width progress for B3-900	86
Figure (4-5a) Load deflection curve for beam (B0-500).....	87

Figure (4-5b) Load deflection curve for beam (B0-700)	88
Figure (4-5c) Load deflection curve for beam (B0-900)	88
Figure (4-5d) Load deflection curve for beam (B1-500)	89
Figure (4-5e) Load deflection curve for beam (B1-700)	89
Figure (4-5f) Load deflection curve for beam (B1-900)	90
Figure (4-5g) Load deflection curve for beam (B2-500)	90
Figure (4-5h) Load deflection curve for beam (B2-700)	91
Figure (4-5i) Load deflection curve for beam (B2-900)	91
Figure (4-5j) Load deflection curve for beam (B3-500)	92
Figure (4-5k) Load deflection curve for beam (B3-700)	92
Figure (4-5l) Load deflection curve for beam B3-900	93
Figure (4-6) Comparison between the Shear Cracking loads and Ultimate loads of tested deep beams	94
Figure (4-7a) Failure mode of B0-500.....	97
Figure (4-7b) Failure mode of B0-700.....	98
Figure (4-7c) Failure mode of B0-900.....	98
Figure (4-8a) Failure mode of B1-500	99
Figure (4-8b) Failure mode of B1-700	99
Figure (4-8c) Failure mode of B1-900	100
Figure (4-9a) Failure mode of B2-500	101
Figure (4-9b) Failure mode of B2-700	101
Figure (4-9c) Failure mode of B2-900	102
Figure (4-10a) Failure mode of B3-500.....	102
Figure (4-10b) Failure mode of B3-700	103
Figure (4-10c) Failure mode of B3-900	103
Figure (4-11) Load versus longitudinal steel strain for beam (B0-500)	105
Figure (4-12) Load versus longitudinal steel strain for beam (B0-700)	106
Figure (4-13) Load versus longitudinal steel strain for beam (B0-900)	107
Figure (4-14) Load versus longitudinal steel strain for beam (B1-500)	108
Figure (4-15) Load versus longitudinal steel strain for beam (B1-700)	109
Figure (4-16) Load versus longitudinal steel strain for beam (B1-900)	110
Figure (4-17) Load versus longitudinal steel strain for beam (B2-500)	111
Figure (4-18) Load versus longitudinal steel strain for beam (B2-700)	112
Figure (4-19) Load versus longitudinal steel strain for beam (B2-900)	113
Figure (4-20) Load versus longitudinal steel strain for beam (B3-500)	114