

بسم الله الرحمن الرحيم





شبكة المعلومات الجامعية التوثيق الالكتروني والميكروفيلم



شبكة المعلومات الجامعية

جامعة عين شمس

التوثيق الالكتروني والميكروفيلم

قسم

نقسم بالله العظيم أن المادة التي تم توثيقها وتسجيلها على هذه الأفلام قد أعدت دون أية تغيرات



يجب أن

تحفظ هذه الأفلام بعيدا عن الغبار المنافلام بعيدا عن الغبار المنافلام بعيدا عن الغبار المنافلام من ٢٠-٠٤% منوية ورطوية نسبية من ٢٠-٤٠ المنافلات ال



بعض الوثائييق الاصلية تالفة



بالرسالة صفحات لم ترد بالاصل

AIN SHAMS UNIVERSITY
FACULTY OF ENGINEERING
IRRIGATION AND HYDRAULIC DEPARTMENT

EFFICIENCY OF CUTOFFS UNDER APRONS OF HYDRAULIC STRUCTURES

BY

AKRAM KARAM SHEHATA

(B. Sc. Civil Engineering, Ain Shams University 1990)

A Thesis

SUBMITTED FOR THE DEGREE OF DOCTOR OF PHILOSOPHY IN CIVIL ENGINEERING

Supervised By

Prof. Dr.

M. EL-NIAZY HAMMAD

Prof. of Irrigation and Hydraulics
Irrigation and Hydraulics Department
Ain shams University

Dr.

M. ADEL EL-GAMMAL

Assoc. Prof. of Soil Mechanics National Research Center Prof. Dr.

M. ABDELLATEEF MOHAMED

Prof. of Irrigation Designs
Irrigation and Hydraulies Department
Ain shams University

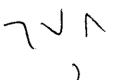
Dr.

ANAS M. EL-MOLLA

Assoc. Prof. of Irrigation and Hydraulics El-Azhar University

Egypt

1996



EFFICIENCY OF CUTOFFS UNDER APRONS OF HYDRAULIC STRUCTURES

Ph.D. Degree Submitted by
AKRAM KARAM SHEHATA
In Civil Engineering (Irrigation and Hydraulics)

EXAMINERS COMMITTEE:

-3.

Signature

- 1. Prof. Dr. M. ELGANAINY

 Professor of Irrigation Structures,
 Irrigation and Hydraulics Department,
 Faculty of Engineering, Alexandria University.
- Prof. Dr. G. SADEK EBAED
 Professor of Irrigation Designs,
 Irrigation and Hydraulics Department,
 Faculty of Engineering, Ain Shams University.
- Prof. Dr. M. EL-NIAZY HAMMAD
 Professor of Irrigation and Hydraulics,
 Irrigation and Hydraulics Department,
 Faculty of Engineering, Ain Shams University.

3. Prof. Dr. M. ABDELLATEEF MOHAMED
Professor of Irrigation Designs,
Irrigation and Hydraulics Department,
Faculty of Engineering, Ain Shams University.

Date: / / 1996

Egypt-1996

ACKNOWLEDGMENTS

الحمد لله رب العالمين

I wish to express my thanks to Prof. Dr. Mohamed El-Niazy Hammad, Professor of Irrigation and Hydraulics, Irrigation and Hydraulics Department, Ain shams University for his constructive advice, valuable discussions and useful revision of this work.

Grateful thanks and deep appreciation are also due to **Prof. Dr. Mahmoud Abdelateef Mohamed,** Professor of Irrigation Structures, Irrigation and Hydraulics

Department, Ain shams University for his helpful advice, great assistance until the completion of this work. Words are not sufficient and can not express my heart feeling towards him.

Deep gratitude and thanks are due to Assoc. Prof. Mohamed Adel El-Gammal, Assoc. Prof. of Soil Mechanics, National Research Center for his kind supervision, gentle attitudes and valuable advice before and during the period of this work.

I would like to express my sincere thanks to my teacher Assoc. Prof. Anas Mohamed El-Molla, Assoc. Prof. of Irrigation and Hydraulics, El-Azhar University for his patience, guidance and continuous help throughout the work.

Special words of thanks to all members of the technical stuff of hydraulic laboratory, Faculty of Engineering, El-Azhar University.

, r - 1

4

ABSTRACT

Akram Karam Shehata. Efficiency of Cutoffs under aprons of hydraulic structures. Doctor of Philosophy in Civil Engineering, Ain Shams University, Faculty of Engineering, Department of Irrigation and Hydraulics.

Weirs, regulators, dams and locks are few examples of hydraulic structures that operate under differential heads between the upstream and downstream sides. The design of such hydraulic structures usually involves a study of the seepage aspects under their aprons. Reduction of the influence of these aspects for the hydraulic structure is generally accomplished by using cutoffs at the two edges of its apron. When the cutoffs are used, their effect on the stability of the structure should be carefully taken into consideration.

In this study, an analytical solution has been obtained for the problem of seepage flow underneath hydraulic structures provided with two unequal cutoffs at the edges of their aprons. The structures are founded on anisotropic layer of varying thickness or stratified soil of two or three layers. The derive **d** equations have been used for computing the seepage characteristics. To facilitate the use of these equations, design charts have been obtained to calculate the relative uplift pressure distribution through the subsurface contour of the structures, the exit gradient at the downstream edge of the structure and the quantity of seepage discharge which seeps under the aprons can be calculated from the relevant charts. Also, a computer program was designed especially to compute the data required for a wide range of structure dimensions. The program is friendly used and includes artificial intelligence unit to check the input data.

The analytical results was calibrated using field data under Naga Hammadi Barrage. The results gave good correlation with the piezometric readings under the apron of the barrage. Also, an electrolytic tank model was designed and constructed especially for verification of the analytical results and for finding out the effective depth of the seepage zone. A good agreement between analytical and experimental results was obtained.

1/2

KEY WORDS: SEEPAGE, HYDRAULIC STRUCTURES, CUTOFFS, ANISOTROPIC MEDIA, VARYING THICKNESS, STRATIFIED SOIL.

CONTENTS

CKNOWLEDGMENTS	
ABSTRACT	
CONTENTS	
NOTATIONS	xi
CHAPTER I	1
INTRODUCTION	1
CHAPTER II	6
LITERATURE REVIEW	
2.1 INTRODUCTION	
2.2 SEEPAGE PROBLEM	
2.2.1 Seepage Flow	
2.2.1.1 Darcy's law	
2.2.1.2 Coefficient of permeability	
2.2.2 Seepage Force	15
2.2.2.1 Uplift pressure	13
2.2.2.2 Piping	10
2.3 MAIN METHODS OF SOLVING SEEPAGE PROBLEMS	
2.3.1 Empirical Methods	
2.3.1.1 Bligh's method	
2.3.1.2 Lane's theory	
2.3.2 Experimental Methods	
2.3.2.1 Sand tank models	
2.3.2.2 Electrical models	
2.3.2.3 Hele-Show models	20
2.3.3 Analytical Methods based on Conformal Mapping technique	21
2 3 3 1 Schwartz-Christoffel transformation method	21

2.3.3.2 Fragment method	25
2.3.4 Approximate Methods.	29
2.3.4.1 Graphical flow-net method	29
2.3.4.2 Method of independent variables	30
2.3.5 Numerical Methods	31
2.3.5.1 Finite element method	31
2.3.5.2 Boundary element method	32
2.3.5.3 Stochastic methods	33
2.3.6 On-Site Investigations	33
2.4 PROGRESS OF SEEPAGE SOLUTIONS UNDER APRONS	34
2.4.1 Seepage under Aprons founded on Isotropic Soil	34
2.4.2 Seepage under Aprons founded on Anisotropic Soil	49
2.4.3 Seepage under Aprons founded on Stratified Soil	51
2.4.4 Seepage under Aprons founded on Heterogeneous Soil	54
CHAPTER III	. 56
ANALYTICAL MODEL	. 56
3.1 INTRODUCTION	. 56
3.2 GOVERNING EQUATIONS	. 56
3.3 BOUNDARY CONDITIONS	. 58
3.3.1 First Condition	. 58
3.3.2 Second Condition	
3.3.3 Third Condition	. 60
3.4 ANALYTICAL SOLUTION	. 60
3.4.1 Analytical Solution for First Condition	. 60
3.4.1.1 Along upstream cutoff (fragment A)	. 62
3.4.1.2 Under apron (fragment B)	. 63
3.4.1.3 Along downstream cutoff (fragment C)	. 70
3.4.1.4 Uplift pressure under apron	. 78
3.4.1.5 Exit gradient at end of apron	. 91
3.4.1.6 Seepage discharge	

3.4.2 Analytical Solution for Second Condition	96
3.4.2.1 Fragment A (through heel cutoff)	99
3.4.2.2 Under apron (fragment B)	100
3.4.2.3 Fragment C (through downstream cutoff)	104
3.4.2.4 Uplift pressure under apron	117
3.4.2.5 Relative exit gradient	132
3.4.2.6 Seepage discharge	136
3.4.3 Analytical Solution for Third Condition	139
3.4.3.1 Semi-infinite domain (fragment A)	141
3.4.3.2 Rectangular domain (fragment B)	142
3.4.3.3 Semi-infinite domain (fragment C)	144
3.4.3.4 Uplift pressure under apron	155
3.4.3.5 Relative exit gradient	167
3.4.3.6 Seepage discharge	170
CHAPTER IV	173
EXPERIMENTAL STUDY	173
4.1 INTRODUCTION	173
4.2 DESCRIPTION OF FACILITIES	175
4.3 THE MODEL	175
4.4 SYSTEMS OF CONTROL	176
4.4.1 Bed level Control	176
4.4.2 Electrical current Control	177
4.5 METHOD OF MEASURING AND INSTRUMENTS	177
4.5.1 Measuring of total Potential Drop.	177
4.5.2 Measurements of the potentials.	178
4.6 RANGES OF TESTING PROGRAMS	178
4.6.1 First Program	. 178
4.6.2 Second Program	. 179
4.7 TEST PROCEDURE	. 181
4.8 OBJECTIVES OF THE STUDY	. 182

4.8.1 Observation the depth of the Effective Seepage zone	
4.8.2 Verification of the Analytical Results	
4.8.3 Validity of the Electrodynamic Analogy Model	
CHAPTER V	,
RESULTS OF ANALYTICAL MODEL 187	•
5.1 INTRODUCTION 187	,
5.2 PROCEDURE OF THE RESULTS)
5.3 THE RESULTS	,
5.4 VERIFICATION OF THE RESULTS	;
5.4.1 Verification of the Results using the Experimental Results	;
5.4.2 Calibration of the Results using Field Data 203	;
CHAPTER VI)
ANALYSIS AND DISCUSSION)
6.1 INTRODUCTION)
6.2 EFFECTS OF THE VARIABLES ON SEEPAGE FLOW 209)
6.2.1 Effect of relative depth of heel cutoff on seepage characteristics 210)
6.2.2 Effect of the ratio of cutoff depths on seepage characteristics 210)
6.2.3 Effect of impervious layer slope (αv) on seepage characteristics 214	1
6.2.4 Effect of impervious layer slope (ap) on seepage characteristics 214	1
6.2.5 Effect of the degree of anisotropy (R) on seepage characteristics 218	3
6.2.6 Effect of the relative thickness (12/T) on seepage characteristics 218	3
6.2.7 Effect of the stratified degree (RI) on seepage characteristics 223	3
6.2.8 Effect of the relative thickness (12/T) on seepage characteristics 228	3
6.2.9 Effect of the stratified degree (R2) on seepage characteristics 228	
6.3 EFFECTS OF THE VARIABLES ON CUTOFFS EFFICIENCY 235	
6.3.1 Effect of relative depth of heel cutoff on the efficiency of cutoffs 235	
6.3.2 Effect of the ratio of cutoffs depths on the efficiency of cutoffs 235	
6.3.3 Effect of impervious layer slope ($\alpha \nu$) on the efficiency of cutoffs 238	

6.3.5 Effect of the degree of anisotropy (R) on the efficiency of cutoffs . 243				
6.3.6 Effect of the relative thickness (12/T) on the efficiency of cutoffs 243				
6.3.7 Effect of the stratified degree (R1) on the efficiency of cutoffs 248				
6.3.8 Effect of the relative thickness (13/1) on the efficiency of cutoffs 248				
6.3.9 Effect of the stratified degree (R2) on the efficiency of cutoffs 253				
6.4 DESIGN CHARTS				
6.5 SOLVED EXAMPLES				
6.5.1 First Example 281				
6.5.1.1 The problem281				
6.5.1.2 The solution				
6.5.2 Second Example 284				
6.5.2.1 The problem				
6.5.2.2 The solution				
CHAPTER VII 288				
SUMMARY AND CONCLUSIONS 288				
7.1 SUMMARY				
7.2 CONCLUSIONS 289				
7.2 007020070				
REFERENCES 292				
REFERENCES 292 Appendix A-1				
REFERENCES 292				
REFERENCES 292 Appendix A-1 The elliptical integrals 299				
Appendix A-1 The elliptical integrals 299 Appendix A-2 The analytical results of hydraulic structure founded on anisotropic layer of				
REFERENCES 292 Appendix A-1 The elliptical integrals 299 Appendix A-2				
Appendix A-1 The elliptical integrals 299 Appendix A-2 The analytical results of hydraulic structure founded on anisotropic layer of upstream thickness deeper than the downstream thickness 301				

Appendix A-4

The analytical results of hydraulic structure founded or	n stratified soil	of two layered
system	,,8,,,,,	339
	tell solve	
The analytical results of hydraulic structure founder	d on stratified	soil of three
layered system		
Appendix A-6		
The results of the first experimental program		
Appendix A-7		
The results of the second experimental program	***************************************	382
ARARIC SIIMMADV		