



AIN SHAMS UNIVERSITY
FACULTY OF ENGINEERING

LATERAL TORSIONAL BUCKLING OF TAPERED I-BEAMS

By
ESAM ABDEL-KADER WALLY

B. Sc., Ain Shams University 1986
M. Sc., Ain Shams University 1998

A Thesis
Submitted in partial fulfillment
for the requirements of the degree of
Ph. D. in Structural Engineering

Supervised By

Professor
Adel H. Salem
Former Dean,
Prof. of Steel Structures
Ain Shams University

Professor
Abdelrahim K. Dessouki
Prof. of Steel Structure
Ain Shams University

Lecturer
Sherif M. Ibrahim
Structural Eng. Dept.
Ain Shams University

Cairo
2007

AIN SHAMS UNIVERSITY
FACULTY OF ENGINEERING

Examiners Committee

Name: Esam Abdel- Kader Wally

Title of Thesis: **Lateral Torsional Buckling of Tapered I-Beams**

Degree: Doctor of Philosophy

<u>Committee</u>	<u>Sign.</u>
1. Prof. Dr./ Dimos Polyzois	()
2. Prof. Dr./ Mahmoud Ibrahim Albanna	()
3. Prof. Dr./ Adel Helmy Salem	()
4. Prof. Dr./ Abdelrahim Khalil Dessouki	()

Date: / / 2007

STATEMENT

This dissertation is submitted to Ain Shams University in partial fulfillment for the requirements of the degree of Doctor of Philosophy in Structural Engineering.

The work included in this thesis was carried out by the author in the Structural Engineering Department, Ain Shams University.

No part of this thesis has been submitted for a degree or a qualification at any other University or Institution.

Date: / /2007

Name: Esam Abdel- Kader Wally

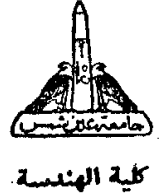
Signature:

بسم الله الرحمن الرحيم

"قالوا سبحانك لا علم لنا إلا ما علمتنا إنك أنت

العليم الحكيم"

صدق الله العظيم



Ain Shams University
Faculty of Engineering
Structural Engineering Department

Abstract of the Ph. D. Thesis submitted by: Eng. Esam Abdel- Kader
Naguib Wally.

Title: **Lateral Torsional Buckling of Tapered I- Beams**

Supervisors:

Prof. Dr. Adel H. Salem

Prof. Dr. Abdelrahim K. Dessouki

Dr. Sherif M. Ibrahim

ABSTRACT

Modern techniques of fabricating plated steel members have resulted in an increased use of tapered elements in steel structures. This is reflected on the material cost by reducing the member cross section in the low moment regions. Tapered beams are fabricated from an arbitrary numbers of flat plates joined by welding along their edges, so that the cross- section consists of a set of connected thin rectangles. When a tapered member does not have an adequate lateral support, its strength is governed by its resistance to torsional buckling modes, one of them is lateral torsional buckling

The present research aims to evaluate the lateral torsional buckling of tapered I- beams. This is achieved by adopting two different techniques; variational approach and finite element method. Variational approach to derive the Euler- Lagrange equations for the displacement components, the equilibrium equations defining instability phenomena and the corresponding limit conditions are obtained for thin, open cross-section, continually tapered beams. The variability of the cross-section along the span introduces an additional term for the expression of the tangential torsions due to variation of the shear centre position. A theoretical analysis was carried out on the basis of the positive definiteness of the second variation of the total potential energy which represents the stability criterion of the beams. Closed form solutions for different cases of tapered beams are presented.

As an alternative analysis method; finite element (F.E) model is presented for tapered I- beams. Material and geometric nonlinearities are incorporated in the F.E. model. Overall geometric imperfection, which causes lateral torsional buckling to I- beams, is incorporated in the model,

as well. Applications are carried out on variable cross-sections of tapered I- beams to calculate critical lateral torsional buckling stresses.

A parametric study is performed on simply supported tapered I-beams with doubly symmetric or mono-symmetric cross sections. Several parameters such as tapering ratios (web tapering, flange tapering), spans, flanges thicknesses, and different types of loading are investigated and their effect on lateral torsional buckling is analyzed.

A set of imperical equations are proposed to predict the lateral torsional buckling load for different types of tapered I- beams.

Finally conclusions and recommendations for future work are presented.

Key words: tapering ratio, symmetric, mono-symmetric, lateral torsional buckling, I-beams.

Acknowledgements

The author would like to express his deep gratitude to professor **Adel Helmy Salem**, former dean, professor of steel structures, Ain Shams University, for his supervision, constant support, and precious advice.

The author is also very grateful to professor **Abdelrahim Khalil Dessouki**, professor of steel structures, Ain Shams University, for his inspiration, encouragement, excellent guidance and endless support through all phases of this research.

The author would like to express his thanks to Dr. **Sherif Mohamed Ibrahim**, lecturer, structural department, Ain Shams University, for his fruitful supervision and friendship throughout the work of this research.

The author offers special thanks to his parents, his wife, his sister, his daughters **Nourhan** and **Maha** for their continuous encouragement and support during the scope of this study.

List of Figures

Figure (2.1) Effect of moment distribution on beam elastic buckling, Trahair and Chan (2002).....	6
Figure (2.2) Effect of load location on beam elastic buckling, Trahair and Chan (2002).....	7
Figure (2.3) Effect of end restraints on beam elastic buckling, Trahair and Chan (2002).....	9
Figure (2.4) Cross section and residual stress, Yoshida (1973).....	10
Figure (2.5) Effect of axial load on elastic buckling, Trahair and Chan (2002).....	15
Figure (2.6) Warping and distortion at an I-section joint, Trahair (1993).....	16
Figure (2.7) Inelastic lateral buckling moments and strengths of beams, Neathercot and Trahair (1976).....	22
Figure (2.8) Flange moments and shears in tapered I-beams Kitipornchai and Trahair (1972).....	29
Figure (2.9) Deflected shape of buckled tapered I-beams, Kitipornchai and Trahair (1972).....	30
Figure (2.10) Flange moments and shears in tapered I-beams, Kitipornchai and Trahair (1975).....	32
Figure (2.11) Geometry of beam-column element: (a) Elevation; (b) Section A-A Bradford and Cuk (1988).....	34
Figure (2.12) Element deformation: (a) Displacements in plane of cross section (b) Flange rotations; (c) Nodal displacements, Bradford and Ronagh (1994).....	37
Figure (2.13) Limiting values of λ_L to enforce second buckling mode, Bradford and Ronagh (1994).....	38

Figure (2.14) Distortional buckling load relative to shear center loading, Bradford and Ronagh (1994).....	39
Figure (2.15) Effect of loading height on distortional buckling, Bradford and Ronagh (1994).....	39
Figure (2.16) Coordinate system and nodal degree of freedom, Chang and Lee (1997).....	41
Figure (2.17) Buckling loads for double- taper simple I- beam under central concentrated load Chang and Lee (1997).....	41
Figure (2.18) Doubly symmetric web tapered I- section beam LRFD (1999).....	42
Figure (2.19 a) Doubly symmetric web tapered I- section beam, Case 1, LRFD (1999).....	43
Figure (2.19 b) Doubly symmetric web tapered I- section beam, Case 2, LRFD (1999).....	44
Figure (2.19 c) Doubly symmetric web tapered I- section beam, Case 3, LRFD (1999).....	45
Figure (2.19 d) Doubly symmetric web tapered I- section beam, Case 4, LRFD (1999).....	45
Figure (3.1) Curvilinear Co-ordinates of general cross section.....	54
Figure (3.2) Angle of rotation characteristics according to polar vectors e & i	55
Figure (3.3) Displacements & rotation of deformed cross section.....	55
Figure (3.4) Vectors a_1, a_2 and a_3 of non- singular point of plane S.....	56
Figure (3.5) (a) Displacement components a_1 and a_3 of point A, (b) Geometric functions of $r(\mu)$ and $q(\mu)$	56
Figure (3.6) Relation between $r(\mu)$ function and $\bar{w}(\mu)$	57

Figure (3.7) External forces and moments distributed on an open cross section beam.....	61
Figure (3.8) Illustrative example of cantilever with tapered I- cross section adopted in μ Coordinates.....	71
Figure (3.9) Distribution of $\bar{\omega}$ and ψ at $L(\xi)$	73
Figure (3.10) Comparison between numerical results: rigidity variation M_{Tl} / ϕ_l with respect to d_L / d_o	75
Figure (3.11) Comparison between numerical results with respect to $\psi = 0$ and $\psi(\mu, \xi)$	77
Figure (3.12) Numerical results: variation of M_{Tl} / ϕ_l with respect to n ...	78
Figure (3.13) Simply supported I-beams under applied end moments....	88
Figure (3.14) Simply supported I-beams with applied concentrated load Q	92
Figure (4.1) Applied moment M_{yl} on the beam at $x = l$, and its affect by rotation angle ϕ	104
Figure (4.2) Doubly symmetric cantilever subjected to external force Q	116
Figure (5.1) Simply supported web tapered I-beam subjected to unequal end moments.....	126
Figure (5.2) Doubly symmetric cantilever subjected to external force Q	131
Figure (5.3) Comparison between results provided by exact solution, closed form equation & finite element method (COSMOS).....	134
Figure (5.4) Relation between critical load and taper ratios for different point of loading (Web- tapered Cantilever Subjected to concentrated load).....	135

Figure (6.1) Nonlinear quadrilateral-thin shell element.....	140
Figure (6.2) Elasto-plastic strain hardening behavior.....	141
Figure (6.3) Simply supported I-beams with applied concentrated load P	143
Figure (6.4) Simply supported doubly symmetric flange tapered I-beam subjected to concentrated load.....	144
Figure (6.5) Original and buckled shapes of different types of tapered I- beams.....	147
Figure (6.6) Stress diagram of maximum stresses on tapered I- beam in lateral torsional buckling mode.....	148
Figure (6.7) Simply supported doubly web tapered I-beam subjected to concentrated load at mid- span.....	149
Figure (6.8) Effect of slenderness ratio on critical- to- yield moment of tapered I- beams (Top flange loading).....	155
Figure (6.9) Effect of slenderness ratio on critical- to- yield moment of tapered I- beams (C.G. loading).....	156
Figure (6.10) Effect of slenderness ratio on critical- to- yield moment of tapered I- beams (Lower flange loading)	157
Figure (6.11) Effect of web- taper ratio on critical- to- yield moment of simply supported tapered I- beams ($\frac{d_o}{t_f} = 30.77$).....	158
Figure (6.12) Effect of web- taper ratio on critical- to- yield moment of simply supported tapered I- beams ($\frac{d_o}{t_f} = 26.67$).....	159
Figure (6.13) Effect of web- taper ratio on critical- to- yield moment of simply supported tapered I- beams ($\frac{d_o}{t_f} = 23.53$).....	160
Figure (6.14) Simply supported doubly- symmetric flange tapered I-beam subjected to concentrated load at mid span.....	170

Figure (6.15) Effect of slenderness ratio on critical- to- yield moment of flange- tapered I- beams (Top flange loading).....	176
Figure (6.16) Effect of slenderness ratio on critical- to- yield moment of flange- tapered I- beams (C.G. loading).....	177
Figure (6.17) Effect of slenderness ratio on critical- to- yield moment of flange- tapered I- beams (Lower flange loading).....	178
Figure (6.18) Effect of flange- taper ratio on critical- to- yield moment of flange- tapered I- beams ($\frac{d_o}{t_f} = 30.77$).....	179
Figure (6.19) Effect of flange - taper ratio on critical- to- yield moment of flange- tapered I- beams ($\frac{d_o}{t_f} = 26.67$).....	180
Figure (6.20) Effect of flange - taper ratio on critical- to- yield moment of flange- tapered I- beams ($\frac{d_o}{t_f} = 23.53$).....	181
Figure (6.21) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 30.77$).....	192
Figure (6.22) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 26.67$).....	193
Figure (6.23) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 23.53$).....	194
Figure (6.24) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 30.77$).....	195
Figure (6.25) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 26.67$).....	196

Figure (6.26) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 23.53$).....	197
Figure (6.27) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 30.77$).....	198
Figure (6.28) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 26.67$).....	199
Figure (6.29) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam ($\frac{d_o}{t_f} = 23.53$).....	200
Figure (6.30) Simply supported doubly symmetric web tapered I-beam with mono- symmetric cross section subjected to concentrated load at mid-span.....	201
Figure (6.31) Effect of slenderness ratio on critical- to- yield moment of mono- symmetric tapered I- beam (Top flange loading).....	205
Figure (6.32) Effect of slenderness ratio on critical- to- yield moment of mono- symmetric tapered I- beam (lower flange loading).....	206
Figure (6.33) Effect of web- taper ratio on critical- to- yield moment of mono- symmetric tapered I- beam ($\frac{d_o}{t_f} = 30.77$).....	207
Figure (6.34) Effect of web- taper ratio on critical- to- yield moment of mono-symmetric tapered I- beam ($\frac{d_o}{t_f} = 26.67$).....	208
Figure (6.35) Effect of web- taper ratio on critical- to- yield moment of mono-symmetric tapered I- beam ($\frac{d_o}{t_f} = 23.53$).....	209
Figure (6.36) Simply supported linearly- varied web- tapered I-beam subjected to unequal end moments.....	218

Figure (6.37) Effect of slenderness ratio on critical- to- yield moment of linearly- varied web- tapered I- beam subjected to end moments ratio, $(\frac{M_1}{M_2} = -0.5)$	222
Figure (6.38) Effect of slenderness ratio on critical- to- yield moment of linearly- varied web- tapered I- beam subjected to end moments ratio, $(\frac{M_1}{M_2} = -1.0)$	223
Figure (6.39) Effect of web- taper ratio on critical- to- yield moment of linearly- varied web- tapered I- beams $(\frac{d_o}{t_f} = 30.77)$	224
Figure (6.40) Effect of web- taper ratio on critical- to- yield moment of linearly- varied web- tapered I- beams $(\frac{d_o}{t_f} = 26.67)$	225
Figure (6.41) Effect of web- taper ratio on critical- to- yield moment of linearly- varied web- tapered I- beams $(\frac{d_o}{t_f} = 23.53)$	226
Figure (6.42) Simply supported linearly- varied flange tapered I-beam subjected to unequal end moments.....	235
Figure (6.43) Effect of slenderness ratio on critical- to- yield moment of linearly- varied flange- tapered I- beam subjected to unequal end moments ratio, $(\frac{M_1}{M_2} = -0.5)$	239
Figure (6.44) Effect of slenderness ratio on critical- to- yield moment of linearly- varied flange- tapered I- beam subjected to unequal end moments ratio, $(\frac{M_1}{M_2} = -1.0)$	240

Figure (6.45) Effect of flange- taper ratio on critical- to- yield moment of linearly- varied flange- tapered I- beams ($\frac{d_o}{t_f} = 30.77$).....	241
Figure (6.46) Effect of flange- taper ratio on critical- to- yield moment of linearly- varied flange- tapered I- beams ($\frac{d_o}{t_f} = 26.67$).....	242
Figure (6.47) Effect of flange- taper ratio on critical- to- yield moment of linearly- varied flange- tapered I- beams ($\frac{d_o}{t_f} = 23.53$).....	243
Figure (6.48) Effect of web/flange tapering on critical- to- yield moment of tapered I- beam subjected to end moment, $\frac{M_1}{M_2} = -0.5$ and ($\frac{d_o}{t_f} = 30.77$)	254
Figure (6.49) Effect of web/flange tapering on critical- to- yield moment of tapered I- beams subjected to end moment, $\frac{M_1}{M_2} = -0.5$ and ($\frac{d_o}{t_f} = 26.67$)	255
Figure (6.50) Effect of web/flange tapering on critical- to- yield moment of tapered I- beams subjected to end moment, $\frac{M_1}{M_2} = -0.5$ and ($\frac{d_o}{t_f} = 23.53$)	256
Figure (6.51) Effect of web/flange tapering on critical- to- yield moment of tapered I- beams subjected to end moment, $\frac{M_1}{M_2} = -1.0$ and ($\frac{d_o}{t_f} = 30.77$)	257
Figure (6.52) Effect of web/flange tapering on critical- to- yield moment of tapered I- beams subjected to end moment, $\frac{M_1}{M_2} = -1.0$ and ($\frac{d_o}{t_f} = 26.67$)	258
Figure (6.53) Effect of web/flange tapering on critical- to- yield moment of tapered I- beams subjected to end moment, $\frac{M_1}{M_2} = -1.0$ and ($\frac{d_o}{t_f} = 23.53$)	259
Figure (B.1) Prismatic beam with general cross section.....	276
Figure (B.2) (a) Displacement components a_1 and a_3 of Point A.(b)	
Geometric Functions of $r(\mu)$ and $q(\mu)$	279