

AIN SHAMS UNIVERSITY FACULTY OF ENGINEERING STRUCTURAL ENGINEERING DEPARTMENT

FLEXURAL BEHAVIOR OF CONCRETE BEAMS REINFORCED WITH HIGH STRENGTH AND CORROSIVE RESISTANT STEEL

Thesis

Submitted in fulfillment of the requirements for the degree of

MASTER OF SCIENCE

In STRUCTURAL ENGINEERING

By

REHAM MOHAMMED GALAL ELTAHAWY

Supervised by

Prof. Sami H. Rizkalla

Distinguished Professor of Reinforced Concrete Structures North Carolina State University

Prof. Osama Hamdy Abdul Wahed

Professor of Reinforced Concrete Structures
Ain Shams University

Dr. Tarek Kamal Hassan

Associate Professor of Reinforced Concrete Structures Ain Shams University

Cairo 2008

EXAMINING COMMITTEE

FOR MASTER OF SCIENCE In STRUCTURAL ENGINEERING

FLEXURAL BEHAVIOR OF CONCRETE BEAMS REINFORCED WITH HIGH STRENGTH AND CORROSIVE RESISTANT STEEL

	Signature
Prof. Mohamed Elsaeed Eissa Professor of Reinforced Concrete Structures Faculty of Engineering – Cairo University	
Prof. Aly Sherif Abdel Fayad Professor of Reinforced Concrete Structures Faculty of Engineering – Ain Shams University	
Prof. Osama Hamdy Abd Elwahed Professor of Reinforced Concrete Structures Faculty of Engineering – Ain Shams University	

Date: 23 February 2008

ACKNOWLEDGEMENT

First of all, thanks to **GOD** and may His peace and blessings be upon all his prophets for granting me the chance and the ability to successfully complete this study.

I wish to express my deepest gratitude to my supervisors **Prof.** Sami Riskallah, Professor of reinforced concrete structures, faculty of engineering, North Carolina State University, whose familiarity with the subject of free speech, and **Prof. Osama Hamdy**, Professor of reinforced concrete structures, faculty of engineering, Ain Shams University, for their valuable advice and guidance of this work. Special thanks and gratitude to Dr. Tarek Hassan, Associate Professor of reinforced concrete structures, faculty of Shams engineering. Ain University. for his genuine valuable advice. sincere comments and deep encouragement through all phases of work which helped me a lot to finish this study in appropriate shape.

This study was partially financed by the North Carolina State University through an undergoing project. I am very grateful to the **authority of North Carolina State University**. I am also grateful to the **authority of Ain Shams University** for allowing me to undertake the present work. I would like to thank all my **Reinforced Concrete Professors** for their support by giving me less work responsibilities to concentrate and finish this study.

My sincere thanks **Mr. Mahmoud Badr, Ahmed and Hani**, technicians at the Reinforced Concrete Lab., Ain Shams University, who co-operated nicely during the construction and assembling of specimens and the tough laboratory work.

I wish to express my gratitude to **Mrs Magda Hanafy**, secretary of Reinforced Concrete Unit, Ain Shams University, for helping me in paper works.

I would like to thank my closest friend, **Haytham Aly**, his support and encouragement especially in the days prior to submitting this study, helped me in completion of this work.

Finally, I am particularly grateful to my mother, **Dalal Amer**, for helping and assisting me in all the stages of this work. Without her help, her sleepless nights and extreme care this study would never have been possible. **The work is dedicated to her, my mother, to whom I owe so much**.

AUTHOR

Name : Reham Mohamed Galal Eltahawy

Date of birth : 2 November 1979

Place of birth : Cairo

Academic Degree: B.Sc. of Civil Engineering

Major : Structural

University : Ain Shams University

Date : June 2002

Grade : Distinction with honor degree

Current job : Teaching and research assistant

FLEXURAL BEHAVIOR OF CONCRETE BEAMS REINFORCED WITH HIGH STRENGTH AND CORROSIVE RESISTANT STEEL

SUMMARY

The research program presented in this thesis was of multi phases to examine the mechanical characteristics of the MMFX steel. their corrosion resistance, investigate and assess evaluate structural performance as main flexural reinforcement using typical full-scale T-section concrete beams reinforced by MMFX rebars. The research presents the experimental program carried out at the Reinforced Concrete Structures laboratory at Ain Shams University, Cairo, Egypt. A total of eight full-scale concrete beams were constructed and tested. All beams were T-section of 500 mm flange width by 80 mm thick. The web was 250 mm wide by 400 mm deep. The nominal length of all beams was 4000mm. Six beams were reinforced by MMFX rebars in the tension side while the remaining two beams were reinforced by conventional steel rebars Grade (40/60) in the tension side. The beams were equally reinforced by conventional steel rebars Grade (40/60) on compression side except for the beam with maximum reinforcement ratio. All beams were tested under static loading conditions to failure in order to investigate the behavior during the pre-cracking, cracking, post-cracking, ultimate capacities, and modes of failure. Deformations, strains in longitudinal reinforcement, and crack pattern were recorded during the test at different loading stages. In the second phase of the research program, a comprehensive analysis of the tested specimens was conducted using cracked section analysis. The predicted behavior was compared to that observed during testing. The design recommendations and guidelines are proposed based on the results from this investigation and additional parametric study.

To fulfill the previously mentioned objectives, this research was divided into the following chapters:

- Chapter (1) is an introduction to this research. This chapter discusses the importance of the research and highlights the scope of the research program.
- Chapter (2) presents a brief review of the available literature, research findings, performance data and current practices relative to the use of MMFX steel in concrete structures.
- Chapter (3) presents the experimental program. This chapter describes the formwork, material properties, fabrication, test specimens, test setup, measurement devices, and loading scheme.
- Chapter (4) presents the test results and observations. The results include load–deflection curves, crack patterns, ultimate capacities, and modes of failure.
- Chapter (5) discusses the methodology adopted in the analytical study and includes verification from experimental work to check the validity of the analytical technique adopted.
- Chapter (6) includes formulation of results into clear design considerations for the use of the high strength and corrosive resistant steel as main flexural reinforcement to be incorporated in the Egyptian Code of Practice.
- Chapter (7) highlights the general conclusions of the study alongside with recommendations for future research and developments in the subject.

Flexural Behavior of Concrete Beams Reinforced with High Strength and Corrosive Resistant Steel

Submitted By

Eng. Reham Mohamed Galal

A thesis Submitted to the Graduate Faculty of Engineering, Ain Shams
University In partial fulfilment of the requirements for the
Degree of Master of Science

October, 2007

Thesis Summary

Use of Micro-composite Multi-structural Formable Steel, commercially known as "MMFX", as a replacement for convention steel is gaining popularity in many concrete structures. The high-corrosive resistance nature and high-strength characteristics of the MMFX rebars could provide additional service life to concrete structures in areas that are prone to severe environmental conditions.

The research program presented in this thesis was designed to study the flexural behavior of T-section concrete beams reinforced by MMFX rebars. The research presents the experimental program carried out at the Reinforced Concrete Structures laboratory at Ain Shams University, Cairo, Egypt. A total of eight full-scale concrete beams were constructed and tested. All beams were T-section of 500 mm flange width by 80 mm thick. The web was 250 mm wide by 400 mm deep. The nominal length of all beams was 4000mm. Six beams were reinforced by MMFX rebars in the tension side while the remaining two beams were reinforced by conventional steel rebars Grade (40/60) in the tension side. The beams were equally reinforced by conventional steel rebars Grade (40/60) on the compression side. All beams were tested under static

loading conditions to failure in order to investigate the behavior during the precracking, cracking, post-cracking, ultimate capacities, and modes of failure.

Deformations, strains in longitudinal reinforcement, and crack pattern were recorded during the test at different loading stages. In the second phase of the research program, a comprehensive analysis of the tested specimens was conducted using cracked section analysis. The predicted behavior was compared to that observed during testing.

All MMFX reinforced concrete beams experienced higher ultimate strength and a comparable amount of ductility in comparison to beams reinforced by conventional steel rebars Grade (40/60). The failure mode of most beams was classified as ductile flexural failure due to significant straining of the tension reinforcement preceding the crushing of the concrete. Only two beams failed due to rupture of the MMFX rebars. No bond failure was observed during testing. The design recommendations and guidelines proposed based on the results from this investigation and additional parametric Based on the research findings, the minimum and maximum reinforcement ratio has been identified as well as the optimal use of MMFX as reinforcement for concrete structures.

Lami Fatala

Prof. Dr. Sami H. Rizkalla
Distinguished Professor
Civil, Construction and Environmental Engineering Dept.
NC State University
Raleigh, NC, USA

Prof. Dr. Osama Hamdy Abdel Wahed Professor of Concrete Structures Faculty of Engineering Ain Shams University

Dr. Tarek Kamal Hassan

Assistant Professor Structural Engineering Dept. Faculty of Engineering

Ain Shams University

TABLE OF CONTENTS

TAE	BLE O	F CONTENTS
LIST	ΓOFF	iguresiv
LIST	ΓOFT	ABLESvi
LIST	ΓOFE	QUATIONSvii
NOT	ΓΑΤΙΟ	NSiz
ABS	STRAC	CTxi
Chaj	pter 1	14
1.1	Gene	eral 14
1.2	Scop	e of the Research16
1.3	Thes	is Outline 16
Chap	pter 2	18
2.1	Intro	duction 18
2.2	Defin	nition of MMFX21
2.3	Mate	rial Characteristics of MMFX 22
2.	3.1	Corrosion Resistance 22
2.	3.2	Strength 24
2.	3.3	Fatigue 25
2.	3.4	Toughness 26
2.	3.5	Brittleness 26
2.	3.6	Bond Strength 27
2.4	Rein	forcing Product Evaluation 28
2.5	Field	Applications31
2.6	Flexu	ural Behavior of Concrete Beams Reinforced by MMFX rebar 33
Chap	pter 3	40
3.1	Gene	eral40
3.2	Form	n Work 41
3.3	Test	Specimens 42
3.	3.1	Concrete Dimensions of Beams42
3.	3.2	Labeling of Test Specimens 43
3.	3.3	Design of Test Specimens 43
3.	3.4	Flexural and Compression Reinforcement45

3	3.5	Shear Reinforcement	46 -
3.4	Mate	terial Properties	47 -
3.4	4.1	Concrete	47 -
	3.4	4.1.1 Compressive Strength	48 -
	3.4	4.1.2 Tensile Strength	49 -
3.4	4.2	Reinforcement	49 -
	3.4	4.2.1 Conventional Steel Reinforcement	49 -
	3.4	4.2.2 MMFX Rebars	51 -
	3.4	4.2.3 Corrosion Resistance	56 -
3.5	Fabri	rication	61 -
3.6	Instru	rumentation	62 -
3.	6.1	Linear Variable Displacement Transducers	62 -
3.	6.2	Strain Gauges	64 -
3.	6.3	Data Acquisition	65 -
3.7	Testi	ting Procedure	65 -
3.	7.1	Test Setup and Loading Scheme	65 -
3.	7.2	Preparation for testing	67 -
Chap	oter 4.		68 -
4.1	Intro	oduction	68 -
4.2	Type	e of Tensile Reinforcement	69 -
4.	2.1	Flexural Behavior	69 -
4.	2.2	Crack Pattern	73 -
4.	2.3	Failure Mode	77 -
4.3	Flexu	xural Reinforcement ratio	80 -
4.	3.1	Flexural Behavior	80 -
4.	3.2	Crack Pattern	83 -
4.	3.3	Failure Mode	85 -
4.4	Conc	ncrete Compressive Strength	88 -
4.	4.1	Flexural Behavior	88 -
4.	4.2	Crack Pattern	90 -
4.	4.3	Failure Mode	92 -
Chap	oter 5.		95 -
5 1	Intro	oduction	- 95 -

5.2	Flexu	ural Analysis	95 -
5.	2.1	Cracking Moment Calculation	99 -
5.	2.2	Failure Criteria	99 -
5.3	Mate	rial Modeling	99 -
5.	3.1	Concrete	100 -
5.	3.2	Compression Reinforcement	102 -
5.	3.3	Flexural Tensile Reinforcement	103 -
5.4	Defle	ection Prediction	104 -
5.5	Verif	fication of Analytical Model	106 -
5.	5.1	Grade (40/60) Beams	107 -
5.	5.2	MMFX Beams	109 -
Chaj	pter 6		114 -
6.1	Intro	duction	114 -
6.2	Basis	s of Current Design Practice	115 -
6.3	Desi	rable Behavior for Flexural Members	115 -
6.4	Past	and Present ACI 318 Code Requirements	116 -
6.5	Nom	inal Strength Analysis	117 -
6.6	Desig	gn Resistance Factor Guidelines	119 -
Chaj	pter 7		124 -
7.1	Gene	eral	124 -
7.2	Conc	clusions	125 -
7.3	Reco	ommendations for Future Study	127 -
REE	RENC	TES.	_ 129 _

LIST OF FIGURES

Figure 2.1- Accelerated Chloride Threshold (ACT) test on MMFX 23	-
Figure 2.2- Typical stress-strain curve of MMFX and Grade 60 steel in tension 25	-
Figure 2.3- Typical fatigue limit of MMFX and Grade 60 steel 26	-
Figure 2.4- Typical Brittle Fracture Data of MMFX and Grade 60 steel 27	-
Figure 2.5- Linear Regression for No. 4 Bar Specimens 28	-
Figure 2.6- Linear Regression for No. 6 Bar Specimens 28	-
Figure 2.7- Field application of MMFX steel 32	-
Figure 2.8- Load-deflection behavior by Marcus H. Ansley 34	-
Figure 3.1- Formwork 42	-
Figure 3.2- Test specimen — - 43 -	-
Figure 3.3- Typical Reinforcement of Test Specimens 45	-
Figure 3.4- Steel Cages for all beams 46	-
Figure 3.5- Stirrup Detail 46	-
Figure 3.6- Stirrup Spacing Detail 47	-
Figure 3.7- Stress strain behavior for mild steel 50	-
Figure 3.8- Mild Steel Sample 50	-
Figure 3.9- Stress strain behavior for high tensile steel 51	-
Figure 3.10- Conventional Deformed bars 51	-
Figure 3.11- Measured stress-strain behavior for MMFX rebars 55	-
Figure 3.12- MMFX rebars 56	-
Figure 3.13- Corrosion Test Reservoir 56	-
Figure 3.14- Measured stress-strain behavior for Corroded MMFX rebars 58	-
Figure 3.15- Measured stress-strain behavior for Corroded Grade (40/60) rebars 60	-
Figure 3.16- Locations of strain gauges on beams 62 -	-
Figure 3.17- Locations of LVDTs on beams63	-
Figure 3.18- Locations of LVDTs on beams63	-
Figure 3.19- Locations of strain gauges on reinforcement bars 64	-
Figure 3.20- Locations of strain gauges on the concrete surface 64	-
Figure 3.21- Data acquisition device and channel box65	-
Figure 3.22-Test setup for specimens 66	_

Figure 3.23-Test setup for XC1-1	- 67 -
Figure 4.1-Load-midspan deflection behavior for beams	- 70 -
Figure 4.2-Load-steel strain relationship for beams XC1-1 & SC1-1	- 71 -
Figure 4.3-Load-midspan behavior relationship for beams	
Figure 4.4-Load-steel strain relationship for beams XC1-3 & SC1-3	
Figure 4.5-Crack Pattern for beam SC1-1	
Figure 4.6-Crack Pattern for beam XC1-1	
Figure 4.7-Crack Pattern for beam SC1-3	
Figure 4.8-Crack Pattern for beam XC1-3	
Figure 4.9-Load-concrete strain relationship for beams SC1-1, XC1-1, SC1-3 & XC1-3	
Figure 4.10-Failure Mode for beam SC1-1	- 78 -
Figure 4.11-Failure Mode for beam XC1-1	- 78 -
Figure 4.12-Failure Mode for beam SC1-3	- 79 -
Figure 4.13-Failure Mode for beam XC1-3	- 79 -
Figure 4.14-Load-midspan deflection relationship for beams	- 81 -
Figure 4.15-Load-steel strain relationship for beams XC1-1, XC1-2,	- 82 -
Figure 4.16-Crack Pattern for beam XC1-1	- 83 -
Figure 4.17-Crack Pattern for beam XC1-2	- 83 -
Figure 4.18-Crack Pattern for beam XC1-3	- 84 -
Figure 4.19-Crack Pattern for beam XC1-4	- 84 -
Figure 4.20-Load-concrete strain relationship for beams XC1-1, XC1-2, XC1-3 & XC1-4.	- 85 -
Figure 4.21-Failure Mode for beam XC1-1	- 86 -
Figure 4.22-Failure Mode for beam XC1-2	- 86 -
Figure 4.23-Failure Mode for beam XC1-3	- 87 -
Figure 4.24-Failure Mode for beam XC1-4	- 87 -
Figure 4.25-Load-midspan deflection relationship for beams	- 89 -
Figure 4.26-Load-steel strain relationship for beams XC1-3, XC2-3 & XC3-3	- 90 -
Figure 4.27-Crack Pattern for beam XC1-3	- 90 -
Figure 4.28-Crack Pattern for beam XC2-3	- 91 -
Figure 4.29-Crack Pattern for beam XC3-3	- 91 -
Figure 4.30-Load-concrete strain relationship for beams XC1-3, XC2-3 & XC3-3 .	- 92 -
Figure 4.31-Failure Mode for beam XC1-3	- 93 -