SHEAR TRANSFER IN COMPOSITE CONCRETE-CONCRETE T-SECTION

By Abdallh Mostafa Soliman Hassan

A thesis submitted to
the faculty of engineering at Cairo university
in partial fulfillment of the
requirements for the degree of
MASTER OF SIENCE

In construction

Faculty of Engineering, Cairo University

Giza, Egypt

2016

T انتقال قوي القص في القطاعات الخرسانيه على شكل حرف

اعداد عبدالله مصطفى سليمان حسن

رساله مقدمه الي كلية الهندسة جامعة القاهرة كجزء من متطلبات الحصول علي درجة ماجستير العلوم في الهندسة الانشائيه

كلية الهندسه-جامعة القاهره الجيزه- جمهورية مصر العربيه

Acknowledgments

I wish to express my deep appreciation for **Prof. Dr. Mohamed Rabie** for his fatherly guidance and encouragement. I also like to extend my acknowledgments to **Dr. Wael Salah** for his indispensable help, valuable assistance and fruitful discussion.

I am also grateful to **Eng. Radwa Sameh** for her help with practical work. And finally, I am indebted to my family (specially **my mother**), friends and colleagues for their help and encouragement throughout the duration of this research.

Contents

CHA	APTER (1)	
INTI	RODUCTION	1
1.1	General	
1.2	Advantages and disadvantages of composite concrete section	
1.2	2.1 ADVANTAGES	2
1.2	2.2 DISADVANTAGES	2
1.3	Objective	•••••
1.4	Format of thesis	•••••
Ch	hapter (1):-	4
Ch	hapter (2):-	4
Ch	hapter (3):-	4
Ch	hapter (4):	5
Ch	hapter (5):	5
CHA	APTER 2	
REV	VIEW OF PREVIOUS WORK	•••••
2.1 (GENERAL ABOUT COMPOSITE ACTION	
2.2 I	HISTORICAL DEVLOPMENT	
2.3 1	THE PROBLEM OF SHEAR TRANSFER AT INTERACTION	•••••
2.3	3.1 GENERAL TYPES OF INTERFACE CONDITIONS	7
	A) NO INTERACTION	7
	B) FULL INTERACTION	7
	C) PARTIAL INTERACTION	7
	3.2 A COMPARISON BETWEEN COMPOSITE BEAMS HAVING FULL AND ARTIAL COMPOSITE ACTION	8
2.3	3.3 COMMON TYPES OF CONCRETE-CONCRETE COMPOSITE ELEMENTS 1	0
2.3	3.4 METHODS OF SHEAR TRANSFER (TYPES OF CONNECTIONS) 1	2
:	2.3.4.1 ROUGH INTERFACE CONNECTION1	2
:	2.3.4.2 SHEAR KEYED ROUGH INTERFACE	4
	2.3.4.3 STEEL DOWELED SHEAR CONNECTIONS 1	5

2.3.4.3.1 THE SHEAR FRICTION THEORY	15
2.3.4.3.2 GENERAL THEORY OF SHEAR TRANSFER MECHANISM FOR CONCRETE:-	17
2.3.4.3.3 EFFECT OF PERCENTAGE OF SHEAR REINFORCEMENT	19
2.3.4.3.4 DISTRIBUTION OF THE STEEL DOWELS OVER THE SPAN	20
2.3.4.4 EPOXY BINDING MATERIAL	20
2.3.5 Shear Transfer Tests:-	20
2.3.5.1 Direct Shear Test:-	21
2.3.5.2 Composite Beam or Slab Test:-	21
2.3.6 PREDICTION OF ULTIMATE SHEAR STRENGTH OR JOINTS (INTERFACES)	21
2.3.6.1 SHEAR STRENGTH FORMULAE	22
A) EMPRICAL FORMULAE	22
B) THEORETICAL FORMULAE	22
2.4 CODE REQUIERMENTS	23
2.4.1 American Concrete Institute (ACI) (2008)	23
2.4.2 American Association of State Highway and Transportation Officials (AASHTO) (2007)	24
2.4.3 Bs 8110 (1985)	28
2.4.4 ISG 0110/1985	29
2.4.5 DIN 1045 / 1978	31
2.5 EFFECT OF CONCRETE PROPERITIES ON SHEAR TRANSFER AT	
NTERFACE	31
2.5.1 COMPRESSIVE STRENGTH	31
2.5.2 TENSILE STRENGTH	31
2.5.3 Dimensions and Reinforcement of The tested Specimens:-	31
2.5.4 EFFECT OF DIFFERENTIAL SHRINKAGE OF CONCRETE	32
2.5.5 Effect of Slip:-	32
2.5.6 Effect of composite interface position with respect to neutral axis	33
Fig. (2.1) various types of concrete-concrete composite sections.	34
Fig. (2.2) Deflected composite beam and Non composite beam	35
Fig. (2.3) Load slip relationship of MENZIES	36
Fig. (2.5) Shear specimen & shear-slip relationship tested by MOUSSA	37

Fig. (2.6) Test setup of CHARKRABTI	38
Fig. (2.8) Connections tested by HARRY	39
Fig. (2.9) Dimension of specimen tested by HARRY.	40
Fig. (2.10) Different displacement relationship obtained by HARRY	41
Fig. (2.12) Assumptions of shear friction theory.	42
Fig. (2.13) Idealized shear resistance forces.	43
Fig. (2.14) Effect of shear reinforcement on shear transfer capacity.	44
Fig. (2.15) Effect of the factor Q F_y on shear transfer capacity.	45
Fig. (2.16) Direct shear specimens.	46
Fig. (2.17) Different system for applying loads to the joints in shear transfer predict	tion.
Fig. (2.18) Modified Mohr-Columb yield criterion for prediction of the failure of concrete	48
Fig. (2.19) Comparison between the empirical, theoretical and design formulas with experimental data of HAShim.	
Fig. (2.20) Effect of compressive strength and Q F_y on maximum shear stress given HANS.	
Fig. (2.21) Effect of compressive strength and Q \mathbf{F}_y on maximum shear stress given HOFBEC.	-
Fig. (2.22) Strain distribution showing two beams action	52
CHAPTER (3)	53
EXPERIMENTAL WORK AND TEST PROGRAM	53
3.1 General:-	53
3.2 Test samples:-	53
3.2.1 Preparation of test samples:-	53
3.2.1.1 Materials:-	54
3.2.1.1 Concrete mix design:-	54
3.2.1.2 Mixing, placing and Curing:-	55
3.2.1.3 Properties of Hardened Concrete:-	55
3.2.2 Composite Beam specimens:-	56
3.2.2.1 Tested Specimens:-	56
3.2.2.2 Loading location and Loading Rate:-	58
3.2.2.3 Measuring devices:-	58
1. Strain Devices:	58

	2-Deflectmetres:-	58	
	Fig.(3.1) Sieve Analysis of Gravel	59	
	Fig.(3.2) supported specimen	60	
	Fig.(3.3) Concentrated Load at Middle Span	61	
	Fig.(3.4) Dimensions of Samples	62	
	Fig.(3.5) Beam (1), Control Monolithic Beam F _{cu} =250kg/cm ²	63	
	Fig.(3.6) Beam (2), L=6φ, F _{cu} =250kg/cm ²	64	
	Fig.(3.7) Beam (3), L= 10φ, F _{cu} =250kg/cm ²	65	
	Fig.(3.8) Beam (4), L=15 φ, F _{cu} =250kg/cm ²	66	
	Fig.(3.9) Beam (5), Epoxy , F _{cu} =250kg/cm ²	67	
	Fig.(3.10) Beam (6), Roughening , F _{cu} =250kg/cm ²	68	
	Fig.(3.11) Beam (7), L=6 ϕ , F _{cu} =400kg/cm ²	69	
	Fig.(3.12) Beam (8), L= 10φ, F _{cu} =400kg/cm ²	70	
	Fig.(3.13) Beam (9), L=15 φ, F _{cu} =400kg/cm ²	71	
	Fig.(3.14) Beam (10), Epoxy , F _{cu} =400kg/cm ²	72	
	Fig.(3.15) Beam (11), Roughening , F _{cu} =400kg/cm ²	73	
	Fig.(3.16) Points for Strain Measurements for tested beam.	74	
	Fig.(3.17) strain gage.	75	
	Fig.(3.18) Electric strain to measure strain in shear connectors.	76	
	Fig.(3.19) Dial Gage to Measure Deflection of the Beam	77	
	Fig.(3.20) Dial Gage to Measure Slip between Beam and Slab in Y Direction	78	
	Fig.(3.21) Dial Gage to Measure Slip between Beam and Slab in X Direction	79	
CI	HAPTER (4)		80
ΑN	NALYSIS AND DISCUSSION OF TEST RESULTS		80
4.1	General:-		80
4. 2	Result of Tests on Composite Reinforced concrete-concrete T section:		80
4	4.2.1 Crack Pattern and Mode of Failure:	81	
4	4.2.2 Load-Deflection Diagram:-	84	
4	4.2.3 Max Slip at Failure:	85	
4	4.2.4 Load-Max crack width Diagram:	85	
	Fig. (4.1) Cracks pattern of Beam B1	88	
	Fig. (4.2) Cracks pattern of Beam B2	88	

Fig. (4.3) Cracks pattern of Beam B3	89	
Fig. (4.4) Cracks pattern of Beam B4	89	
Fig. (4.5) Cracks pattern of Beam B5	90	
Fig. (4.6) Cracks pattern of Beam B6	90	
Fig. (4.7) Cracks pattern of Beam B7	91	
Fig. (4.8) Cracks pattern of Beam B8	91	
Fig. (4.9) Cracks pattern of Beam B9	92	
Fig. (4.10) Cracks pattern of Beam B10	92	
Fig. (4.11) Cracks pattern of Beam B11	93	
Fig.(4.12) Load Deflection Diagram for Beams	94	
Fig.(4.13) Load Deflection Diagram for Beams	95	
Fig.(4.14) Load Deflection Diagram for Beams	96	
Fig.(4.15) Load Slip Diagram for Beam2	97	
Fig.(4.16) Load Slip Diagram for Beam3	98	
Fig.(4.17) Load Slip Diagram for Beam4	99	
Fig.(4.18) Load Slip Diagram for Beam5	100	
Fig.(4.19) Load Slip Diagram for Beam6	101	
Fig.(4,20) Load Slip Diagram for Beam7	102	
Fig.(4.21) Load Slip Diagram for Beam8	103	
Fig.(4,22) Load Slip Diagram for Beam9	104	
Fig.(4.23) Load Slip Diagram for Beam10	105	
Fig.(4.24) Load Slip Diagram for Beam11	106	
Fig.(4.25) Max Slip at Failure	107	
Fig.(4.26) Variation of Crack Width with Load for F _{cu} =250kg/cm ²	108	
Fig.(4.27) Variation of Crack Width with Load for F_{cu} =400kg/cm ²	109	
Fig.(4.28) Max Crack Width at Failure for all Beams	110	
Fig.(4.29) Ultimate Load at Failure for all Beams	111	
CHAPTER (5)		112
Conclusions		112
5.1 General:-	112	
5.2 Composite Reinforced Concrete Beams:	112	
A. The effect of shear connectors	112	

В.	The effect of Epoxy and Roughening	112
C.	The effect of the concrete strength	113
5.3 Re	ecommendation for further study:	114

LIST OF SYMPOLS

Symbol Description

A_s"Area of steel tics across the plane.

P' Percentage of steel ties across the shear plane.

F_vYield stress of steel ties.

F_c' Concrete compressive strength.

V_uUltimate shear force.

V Ultimate shear strength.

 ΔV_b Shear resistance by intrinsic bond.

 ΔV_f Shear resistance by friction.

 ΔV_i Shear resistance by aggregate Interlock.

 $\Delta V_{\text{d}} Shear$ resistance by dowel action.

C_o Apparent cohesion.

K₁Ratio of bonded area to total area.

μ' Apparent coefficient of friction.

I Reinforcement parameter P" F_v.

C' Apparent cohesion.

C_kTotal area or the shear key cross sections along the composite Interface.

V_mUltimate shear strength of the shear key consider as monolithic.

A_{sh}Area of the contact surface between two concretes.

q_cUltimate shear stress carried by the interface due to cohesion forces.

vpoisson ratio.

G Shear modules

ABSTRACT

The problem of shear transfer between different types of concrete surface cast at different ages has been discussed in many researches. Different types of surface treatments and shear connectors were studied experimentally to test their efficiency in what is called the (composite action) which could be formed between a precast beams and cast in place slabs. In this research 11 composite concrete-concrete T-section with dimensions (beams 120*400*2000mms and slabs 500*100*2000mms) were tested under static concentrated load in the middle of the span, one of them was reference monolithic T-section and the others were with different types of surface treatment like epoxy and roughening, Also shear connectors with different lengths, and increasing concrete strength were studied to obtain the best way to transfer the shear between concrete layer

CHAPTER (1)

INTRODUCTION

1.1 General

The composite concrete structures have been always a topic of major importance in the field of construction. The use of concrete in composite structures may be with steel sections in the form of steel-concrete composite sections, or with concrete cast at different times to form what is called concrete-concrete composite sections which are used in buildings and bridge constructions. Economic considerations indicate the desirability of composite action between the precast and the cast in place elements. Composite behaviour, with the slab serving as the flange of T-beam results in horizontal shearing stress at the construction joints. The stresses at this region are very complicated and several trials to represent the behaviour in this region have been made.

The rapid population growth in Egypt is increasing the need for housing projects, multistory garages and also for bridges rapidly constructed. Precast concrete with casts in place slab has this advantage. There is an urgent need for further investigations, laboratory tests, numerical analysis and theoretical studies to find an acceptable method for solving the problem of shear transfer along the interface between precast concrete and cast-in-place concrete to simplify dealing with such structures as the need for them is becoming urgent.

1.2 Advantages and disadvantages of composite concrete section

In concrete-concrete composite structures the precast beam acts as a form for the cast in place slab. According to some economic considerations for the application of such structures some of the advantages and the problems of this type of construction are summarized as following:

1.2.1 ADVANTAGES

- 1) Quality control on precast beams can be easily made as they are manufactured in proper conditions. The concrete can be made denser, more resistant to corrosion, less permeable, stronger and of more uniform quality than the concrete cast-in-place in the field improved quality of concrete should result in lower maintenance and repair cost and longer service life for the structures.
- 2) Using precast beams as a form for cast-in-place concrete reduces cost as it eliminates the need for forming which represents in some cases a high percentages of the cost.
- 3) The precast concrete could be designed as a form to support the weight of all the wet concrete above, thus form to support scaffolding can be eliminated and this could solve lots of traffic problems in Egypt during bridges construction.
- 4) The time required for construction can be reduced by the use of precast concrete integrated formwork as stated, and this also could help solving lots of construction time in Egypt.

1.2.2 **DISADVANTAGES**

- 1) Problems with precast beams construction, as extra reinforcement is needed to withstand handling stresses and the problem of storing precast units to provide mass production.
- 2) Handling and transportation of the precast units may require special details as well as special precast form units.
- 3) Special propping may be needed to support the precast beam during casting. In addition to, the problem of differential shrinkage between the precast element and the cast in place concrete should be taken into account.

4) Treatment of joints along the interface before and after pouring the cast-in-place concrete is the main problem that could face such structures.

Problems of composite action of the beam and slab together, impermeability of joints and good appearance of concrete face finish.

The least stated problem made many researchers investigate this subject to ensure full composite action at the interface for both precast beams and cast-in-place slabs, these researchers concerned mainly with the problem of shear transfer along the interface. The shear force must be transmitted across the interface between the two layers in the same manner and with the same deformations as if the entire section was casting monolithically.

Roughness, intermediate roughness, steel dowels, shear keys, steel angles, epoxy binding......etc., may be used to improve the shear transfer.

1.3 Objective

The main objectives of this research can be summarized as follows:-

- Studying the shear transfer across interface in composite concreteconcrete T-section.
- Get the best way of treatment the interface to achieve maximum load transfer, minimum deflection, minimum slip and minimum crack width
- Studying the actual behaviour of simply supported T-section beam under concentrated loads.

1.4 Format of thesis

The present thesis consists of five chapters:-

Chapter (1):-

Presents a general idea about the development of using precast concrete with casting in place concrete to form the composite concrete-concrete section. It also contains the advantages and disadvantages of this type of constructions according to some economic considerations. The chapter also includes the objectives and format of the thesis.

Chapter (2):-

Presents a review of available pervious work concerning composite action of composite beams, precast beams with casting in place slabs. Also, different types of shear connectors and factors affecting their strength are reviewed. It also presents method of shear transfer across an interface.

Chapter (3):-

Presents the experimental work and test program which consist of testing for 11 reinforced composite concrete-concrete T- section, one of them was reference monolithic T-section and the others ten were composite precast beams and cast in place slabs under the effect of concentrated load at the mid span of the beam using different types of interface connections. The behavior of those sections studied under the effect of the following parameters:-

- Using to shear connectors with different lengths.
- Roughening the interface to a higher degree of roughness.
- Using bending materials (Epoxy).
- Using different concrete strength.