

# FACULTY OF ENGINEERING STRUCTURAL ENGINEERING DEPARTMENT

# STRENGTHENING OF T-JOINTS COMPOSED OF CHS USING THROUGH BOLTS

By

Mohamed Abdel Hamid Abdel Wahab Mohamed

### A Thesis Submitted in Partial Fulfillment for the Requirements of the Degree of Master of Science in Civil Engineering (Structural)

Under the supervision of

Dr. Amr Abdel Salam Shaat
Associate Professor
Structural Engineering department
Faculty of Engineering
Ain Shams University

Prof. Ezzeldin Yazeed Sayed-Ahmed
Professor
Structural Engineering department
Faculty of Engineering
Ain Shams University

## **Examiners committee:**

Prof. Sherif Mourad	(	)
Professor and Dean		
Faculty of Engineering		
Cairo University		
Prof. Emam Soliman	(	)
Professor		
Structural Engineering department		
Faculty of Engineering		
Ain Shams University		
Prof. Ezzeldin Yazeed	(	)
Professor		
Structural Engineering department		
Faculty of Engineering		
Ain Shams University		

# AIN SHAMS UNIVERSITY FACULTY OF ENGINEERING STRUCTURAL ENGINEERING DEPARTMENT

Thesis submitted by: Mohamed Abdel Hamid Abdel Wahab Mohamed. Thesis title: Strengthening of T-joints composed of CHS using Through Bolts. Supervisors: Dr. Amr Abdel Salam Shaat ( ) Associate Professor Structural Engineering department Faculty of Engineering Ain Shams University Prof. Ezzeldin Yazeed Sayed-Ahmed ( ) Professor Structural Engineering department Faculty of Engineering Ain Shams University Registration date: Examination date:

October-2014

March-2012

#### **ABSTRACT**

Increasing the joint capacity of tubular steel structural members has always been a challenge for designers because of their closed nature. The closed nature of these circular hollow sections does not allow conventional stiffeners welding. As such, the present numerical research investigates the efficiency of a new technique for strengthening the circular chord members of T-joints against outside deformation (ovalization) when subject to brace member's compression. The strengthening technique is based on bracing the chord member by through bolts in the radial direction which are arranged in different patterns. A parametric study is carried-out in order to investigate the effects of the number (n) and spacing (s) of through bolts, the chord diameter and the brace-to-chord diameter ratio  $(\beta)$  on the strength gained by the joint using through bolts strengthening technique. Two groups of specimens are chosen; the first group has a chord member of diameter  $(d_0)$  of 356 mm, and brace members diameters (d<sub>1</sub>) of 324 mm, 273 mm and 219 mm, while the second group has a chord member of diameter (d<sub>0</sub>) of 610 mm, and brace members diameters (d<sub>1</sub>) of 559 mm, 508 mm and 406 mm. The study mainly revealed that the percentage gain in both strength and stiffness is directly proportional to the number of through bolts. The study indicates also that the efficiency of this strengthening technique increases with small chord diameters and large  $\beta$ -ratios: e.g. a gain in strength of 47% is achieved for  $\beta$  of 0.91 strengthened with five bolts spaced at 75 mm. The study also revealed that for large bolts' spacing, an odd number of bolts with a central one is more efficient in strengthening the CHS T-joints.

### **ACKNOWLEDGMENTS**

The author would like to express his sincere gratitude and appreciation to his supervisor, Associate Professor Dr. Amr Abdel Salam Shaat, for his priceless supervision, support and encouragement which is an essential role in bringing this research to an end.

The author would also like to send his deepest regards to his supervisor, Prof. Ezzeldin Yazeed Sayed-Ahmed for his guidance and valuable assistance throughout all the stages of the research.

The author would like to thank his family for their relentless support and encouragement to finish this research. He would like to show his gratitude to his late uncle for his continuous encouragement. **STATEMENT** 

This dissertation is submitted to Ain Shams University for the degree

of Master of Science in Civil Engineering (Structural Engineering).

The work included in this dissertation has been carried out by the

author in the Department of Structural Engineering, Ain Shams

University, from March 2012 to October 2014.

No part of this dissertation has been submitted for a degree or a

qualification at any other university or institution.

Date:

October 2014.

Signature:

Name: Mohamed Abdel Hamid Abdel Wahab Mohamed.

vi

## TABLE OF CONTENTS

ABSTRACT	iv
ACKNOWLEDGMENTS	v
STATEMENT	vi
Table of contents	vii
List of figures	X
LIST OF TABLES	xviii
NOTATIONS	xix
CHAPTER 1: INTRODUCTION	2
1.2 Strengthening techniques of HSS joints	
1.2.1 Can method	
1.2.2 Ring stiffeners	
1.2.3 Concrete grouting	
1.2.4 Doubler plates	
1.2.5 Collar plates	
1.2.6 Through bolts	8
1.3 Chord ovalization	9
1.4 Thesis objectives	10
1.5 Thesis contents	10
CHAPTER 2: LITERATURE REVIEW2.1 Introduction	12 12
2.2 Unstiffened joints	12
2.2.1 Experimental studies	
2.2.2 Numerical studies	
2.3 Stiffened joints	
2.3.1 Experimental studies	
2.3.2 Numerical studies	
CHAPTER 3: FINTIE ELEMENT MODELING AND	
VERIFICATION	40
3.1 Introduction	
3.2 Element types	40

3.2.1	Shell elements	. 40
3.2.2	Link elements	. 41
3.3 Ma	terial modeling	. 42
	del verification	
3.4.1	Zhao and Hancock (1991)	. 45
	Choo et al. (2005)	
	Aguilera et al. (2012)	
3.5 Ver	rification results	
CHAPTER 4.1 Intr	R 4: PARAMETRIC INVESTIGATIONS	
4.2 Mo	del description	. 52
4.2.1	Shell size and yield strength	. 53
4.2.2	Through bolts	. 54
4.2.3	Weld modeling	. 54
4.2.4	Specimens description	. 54
4.2.5	T F	
4.2.6	Brace member length (L <sub>1</sub> )	
4.2.7	Classification of chord and brace members	. 55
4.2	.7.1 Chord members	. 55
4.2	.7.2 Brace diameters	. 56
4.2.8	Radius-to-thickness ratio (γ)	. 56
4.2.9	Chord length-to-radius ratio (α)	. 56
4.2.10	Brace-to-chord diameter ratio (β)	. 57
4.3 Par	ametric study	. 57
4.3.1	Group 1 specimens	. 57
4.3.2	Group 2 specimens	. 57
4.3.3	Number of through bolts (n)	. 58
4.3.4	Spacing between through bolts (s)	. 58
4.4 Spe	ecimens designation system	. 58
CHAPTER 5.1 Bel	R 5: RESULTS AND DISCUSSIONSavior of strengthened joints	
5.2 Effe	ect of number of through bolts	. 77
5.2.1	Group 1 Specimens	. 77

5.2.2 Group 2 specimens	77
5.3 Effect of arrangement of through bolts	81
5.3.1 Effect of spacing between through bolts	82
5.3.1.1 Group 1 ( $\beta = 0.91$ )	82
5.3.1.2 Group 1 ( $\beta = 0.77$ )	83
5.3.1.3 Group 1 ( $\beta = 0.62$ )	84
5.3.1.4 Group 2 ( $\beta = 0.92$ )	85
5.3.1.5 Group 2 ( $\beta = 0.83$ )	86
5.3.1.6 Group 2 ( $\beta = 0.67$ )	87
5.3.2 Effect of central bolt	89
5.4 Effect of chord diameter	90
5.4.1 Small β-ratios ( $\beta = 0.62$ )	92
5.4.2 Large $\beta$ -ratios ( $\beta = 0.92$ )	96
5.5 Effect of β-ratio	97
5.6 Failure modes	99
5.6.1 Unstiffened joints	99
5.6.2 Strengthened joints	103
5.6.2.1 Group 1 specimens	103
5.6.2.2 Group 2 specimens	110
CHAPTER 6: CONCLUSIONS	
6.1 Overview	
6.2 Conclusions	115
6.3 Recommendation for future work	118
LIST OF REFERENCES	119
APPENDIX A	131

### LIST OF FIGURES

Figure 1-1: Different HSS joint types
Figure 1-2: Can method5
Figure 1-3: Ring stiffeners6
Figure 1-4: Concrete grouting
Figure 1-5: Doubler plate7
Figure 1-6: Collar plate 8
Figure 1-7: Through bolts9
Figure 1-8: Chord ovalization9
Figure 3-1: Shell element geometry
Figure 3-2: Bar element geometry
Figure 3-3: Stress-strain curves for test specimens adopted from Aguilera
et al. (2012)
Figure 3-4: Stress-strain curves for steel materials
Figure 3-5: Verification model for Zhao and Hancock (1991)45
Figure 3-6: Comparison between numerical and experimental failure
shape of specimen Ex-01 (Choo et al. 2005)
Figure 3-7: Comparison between numerical and experimental failure
shape of specimen Ex-10 (Choo et al. 2005)
Figure 3-8: Comparison between numerical and experimental load
ovalization curves of specimen Ex-01 (Choo et al. 2005)
Figure 3-9: Comparison between numerical and experimental load
ovalization curves of specimen Ex-10 (Choo et al. 2005)
Figure 3-10: Comparison between failure modes of the FEM and the
experiments for specimen T1 (Aguilera et al. 2012)50
Figure 3-11: Comparison between failure modes of the FEM and the
experiments for specimen T2 (Aguilera et al. 2012)
Figure 4-1: Geometry and finite element modeling specimens
Figure 4-2: Schematic drawing. 53

Figure 4-3: Number and configuration of through bolts
Figure 5-1: Studied parameters. 62
Figure 5-2: Load ovalization curves for Group 1 specimens with71
Figure 5-3: Load ovalization curves for Group 1 specimens with 71
Figure 5-4: Load ovalization curves for Group 1 specimens with 72
Figure 5-5: Load ovalization curves for Group 2 specimens with
Figure 5-6: Load ovalization curves for Group 2 specimens with
Figure 5-7: Load ovalization curves for Group 2 specimens with
Figure 5-8: Percentage gain in stiffness for Group 1 specimens with $\beta$ -
ratio = 0.91
Figure 5-9: Percentage gain in stiffness for Group 1 specimens with $\beta$ -
ratio = 0.77
Figure 5-10: Percentage gain in stiffness for Group 1 specimens with $\beta$ -
ratio = 0.62
Figure 5-11: Percentage gain in stiffness for Group 2 specimens with $\beta$ -
ratio = 0.92
Figure 5-12: Percentage gain in stiffness for Group 2 specimens with $\beta$ -
ratio = 0.83
Figure 5-13: Percentage gain in stiffness for Group 2 specimens with $\beta$ -
ratio = 0.67
Figure 5-14: Effect of number of through bolts for Group 1 specimens (β
= 0.91)
Figure 5-15: Effect of number of through bolts for Group 1 specimens (β
= 0.77)
Figure 5-16: Effect of number of through bolts for Group 1 specimens ( $\beta$
= 0.62)
Figure 5-17: Effect of number of through bolts for Group 2 specimens ( $\beta$
= 0.92)

Figure 5-18: Effect of number of through bolts for Group 2 specimens (β
= 0.83)
Figure 5-19: Effect of number of through bolts for Group 2 specimens ( $\beta$
= 0.67)
Figure 5-20: Effect of spacing between bolts for Group 1 with
Figure 5-21: Effect of spacing between bolts for Group 1 with
Figure 5-22: Effect of spacing between bolts for Group 1 with
Figure 5-23: Effect of spacing between bolts for Group 2 with
Figure 5-24: Effect of spacing between bolts for Group 2 with
Figure 5-25: Effect of spacing between bolts for Group 2 with
Figure 5-26: Effect of removing the central bolt in Group 1 with 90
Figure 5-27: Comparison between percentage gain in strength and
stiffness for Group 2 specimens $\beta = 0.92$ (75 mm)
Figure 5-28: Comparison between percentage gain in strength and
stiffness for Group 2 specimens $\beta = 0.92 \ (100 \ mm).$
Figure 5-29: Comparison between percentage gain in strength and
stiffness for Group 2 specimens $\beta = 0.92$ (150 mm)
Figure 5-30: Comparison between percentage gain in strength for different
chord sizes ( $\beta$ = 0.62 and s = 25 mm)
Figure 5-31: Comparison between percentage gain in strength for different
chord sizes ( $\beta$ = 0.62 and s = 50 mm)
Figure 5-32: Comparison between percentage gain in strength for different
chord sizes ( $\beta$ = 0.62 and s =75 mm)
Figure 5-33: Comparison between percentage gain in strength for different
chord sizes ( $\beta$ = 0.62 and s =100 mm)
Figure 5-34: Comparison between percentage gain in strength for different
chord sizes ( $\beta$ = 0.62 and s =150 mm)
Figure 5-35: Comparison between percentage gain in strength for different
chord sizes ( $\beta = 0.62$ and s = 200 mm)

Figure 5-36: Comparison between percentage gain in for different chord
sizes ( $\beta$ = 0.92 and s =75 mm)
Figure 5-37: Comparison between percentage gain in stiffness for
different chord sizes ( $\beta = 0.92$ and s =75 mm)
Figure 5-38: Effect of $\beta$ -ratios on the gain in strength for different bolts
configuration spaced at 75 mm
Figure 5-39: Effect of $\beta$ -ratios on the gain in stiffness for different bolts
configuration spaced at 75 mm
Figure 5-40: Von Misses stresses for specimen 356-324-0-000
Figure 5-41: Von Misses stresses for specimen 356-273-0-000
Figure 5-42: Von Misses stresses for specimen 356-219-0-000 101
Figure 5-43: Von Misses stresses for specimen 610-559-0-000 101
Figure 5-44: Von Misses stresses for specimen 610-508-0-000 102
Figure 5-45: Von Misses stresses for specimen 610-406-0-000 102
Figure 5-46: Von Misses stresses for specimen 356-219-1-000 103
Figure 5-47: Von Misses stresses for specimen 356-273-1-000 104
Figure 5-48: Von Misses stresses for specimen 356-324-1-000 104
Figure 5-49: Von Misses stresses for specimen 356-219-2-075 105
Figure 5-50: Von Misses stresses for specimen 356-273-2-075 105
Figure 5-51: Von Misses stresses for specimen 356-324-2-075 106
Figure 5-52: Failure mode for specimen 356-219-3-200
Figure 5-53: Failure mode for specimen 356-273-3-200
Figure 5-54: Failure mode for specimen 356-324-3-200
Figure 5-55: Failure mode for specimen 356-219-4-050
Figure 5-56: Failure mode for specimen 356-273-4-050
Figure 5-57: Failure mode for specimen 356-324-4-050
Figure 5-58: Von Misses stresses for specimen 610-406-1-000
Figure 5-59: Von Misses stresses for specimen 610-508-1-000 111
Figure 5-60: Von Misses stresses for specimen 610-559-1-000 111