

بسم الله الرحمن الرحيم



-C-02-50-2-





شبكة المعلومات الجامعية التوثيق الالكتروني والميكرونيلم





جامعة عين شمس

التوثيق الإلكتروني والميكروفيلم

قسم

نقسم بالله العظيم أن المادة التي تم توثيقها وتسجيلها علي هذه الأقراص المدمجة قد أعدت دون أية تغيرات



يجب أن

تحفظ هذه الأقراص المدمجة يعيدا عن الغيار







بالرسالة صفحات لم ترد بالأصل











ANALYSIS OF POUNDING BETWEEN TWO BUILDINGS DURING AN EARTHQUAKE

By

MOHAMMED ATTA AHMED ALNAQIR

B.Sc. Civil Engineering, El-Mansoura University

A THESIS

Submitted in partial fulfillment of the requirements for the degree of Master Science

In

Structural Engineering

SUPERVISORS

Prof. Dr. Salah El-Din E. El-Metwally

Structural Engineering Department Faculty of Engineering El-Mansoura University

Assoc. Prof. Dr. Ahmed Mahmoud Yousef

Structural Engineering Department Faculty of Engineering El-Mansoura University

: Analysis of Pounding Between Two Buildings During an Earthquake. Thesis Title

Candidate Name: Mohammed Atta Ahmed Alnagir

Supervisors:

Name	Position	Signature
1- Prof. Dr. Salah El-Din E. El-Metwally	Professor of Concrete Struct, Structural Engineering Dept., Faculty of Engineering, El-Mansoura University.	S. S. Metwally
2- Assoc. Prof. Dr. Ahmed Mahmoud Yousef	Associate Professor, Structural Engineering Dept., Faculty of Engineering, El-Mansoura University.	A

CONTENTS

	·	Page
ACK	NOWLEDGEMENT	iv
	TRACT	
		. V
СНА	PTER 1: INTRODUCTION	
1.1	GENERAL	1
1.2	OBJECTIVES AND SCOPE	
1.3		
СНАІ	PTER 2: ANALYSIS OF POUNDING PROBLEM AND	
	LITERATURE REVIEW	
2.1	INTRODUCTION	4
2.2	OBSERVATIONS OF BUILDING POUNDING DAMAGE	
•	DURING PREVIOUS EARTHQUAKES	5
	2.2.1 General Observations	
•	2.2.2 Building Pounding Damage During 1989 Loma Prieta	
	Earthquake	6
2.3	ANALYSIS OF POUNDING PROBLEM	8
	2.3.1 Equation of Motions	8
	2.3.2 Theory of Pounding Problem	10
	233 Kay Aggumations	10
	2.3.4 Formulation of D. II. D. II.	12
2.4	SUPER-ETABS (THREE DIMENSIONAL ANALYSIS OF	
	BUILDING SYSTEMS	17
2.5	LITERATURE REVIEW ON POUNDING ANALYSIS	19

CHAPTER 3: PARAMETRIC STUDY ON POUNDING PROBLEM

3.2 3.3 3.4	2 CONFIGURATION OF CASE STUDY BUILDINGS	
		2
3.4	3 SELECTED EARTHQUAKE GROUND MOTIONS	24
	4 OVERALL PERFORMANCE OF POUNDING EXAMPLE 1	2
3.5	5 EXAMINATION OF POUNDING PARAMETERS	20
	3.5.1 Effect of Mass	26
	3.5.2 Effect of Structural Stiffness	28
•	3.5.3 Effect of Building Height	29
	3.5.4 Effect of Contact Spring Stiffness	3(
CHA	PTER 4: EFFECT OF GAP SIZE ON STRUCTURAL	
	POUNDING	
4.1	INTRODUCTION	31
		21
4.2	RELATIVE DISPLACEMENT OF ADJACENT BUILDINGS	31
4.2 4.3	Store Mediatry of Adjacent Buildings	31
	BUILDINGS CONSIDERED	32
4.3	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING	32 33
4.3	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior	32 33 34
4.3	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior 4.4.2 Effect of Gap Size on the Drift Behavior	32 33 34 35
4.3	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior 4.4.2 Effect of Gap Size on the Drift Behavior 4.4.3 Effect of Gap Size on the Shear Behavior	32 33 34 35 35
4.3	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior 4.4.2 Effect of Gap Size on the Drift Behavior 4.4.3 Effect of Gap Size on the Shear Behavior 4.4.4 Effect of Gap Size on the Story Acceleration	32 33 34 35 35 37
4.3	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior 4.4.2 Effect of Gap Size on the Drift Behavior 4.4.3 Effect of Gap Size on the Shear Behavior	32 33 34 35 35 37
4.3 4.4	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior 4.4.2 Effect of Gap Size on the Drift Behavior 4.4.3 Effect of Gap Size on the Shear Behavior 4.4.4 Effect of Gap Size on the Story Acceleration	32 33 34 35 35 37
4.3 4.4	BUILDINGS CONSIDERED EFFECT OF GAP SIZE ON THE POUNDING 4.4.1 Effect of Gap Size on the Displacement Behavior 4.4.2 Effect of Gap Size on the Drift Behavior 4.4.3 Effect of Gap Size on the Shear Behavior 4.4.4 Effect of Gap Size on the Story Acceleration 4.4.5 Effect of gap Size on the Story Acceleration	32 33 34 35 35 37

5.2 CODES PROVISIONS FOR GAP SIZE TO AVOID POUNDING	4(
5.2.1 The Egyptian Code for Loads (ECL-93)	
5.2.2 Uniform Building Code (UBC-94)	
5.2.3 Eurocode 8 (EC-8)	4]
5.3 BUILDINGS CONSIDERED	42
5.4 SELECTION OF EARTHQUAKE GROUND MOTIONS	12
5.4.1 Generating of Artificial Earthquakes	13
5.4.2 Selected Time Histories	15
5.5 EVALUATION OF GAP SIZE REQUIRED BY THE CODES	46
CHAPTER 6: CONCLUSIONS AND RECOMMENDATIONS 6.1. CONCLUSIONS	
6.1 CONCLUSIONS	ł8
6.2 RECOMMENDATIONS FOR FUTURE RESEARCH 5	0
REFERENCES 5	1
TABLES	7
FIGURES 66	
APPENDIX 109	5

ACKNOWLEDGEMENT

I wish to express my sincere thanks and appreciation to Prof. Dr. Salah El-Din E. El-Metwally and Assoc. Prof. Dr. Ahmed Mahmoud Yousef, Structural Engineering Department, Faculty of Engineering, El-Mansoura University, for their supervision, encouragement, support and valuable suggestions during all stages of this work.

I would like also to express my gratefulness to all members of my family, for their unconditional efforts and help during the preparation of this work.

ABSTRACT

'Structural pounding refers to the lateral collisions of adjacent buildings. Pounding occurs when building separations are insufficient to accommodate the relative motion of adjacent buildings. Review of past earthquake damage shows that pounding of adjacent buildings during earthquakes is a frequent phenomenon and an important cause of damage and in some cases it has led to collapse.

A formation and solution of the multi-degree-of-freedom equation of motion for pounding between two multistory buildings is presented. A numerical study of the parameters affecting the pounding between adjacent reinforced concrete multistory buildings has been carried out. A dynamic analysis has been utilized to investigate the pounding behavior of various combinations of adjacent reinforced concrete building structures subjected to ground accelerograms. Five examples of pounding problem have been considered in this study. Each example consists of a couple of two reinforced concrete buildings of different heights. These examples included seven different reinforced concrete building structures modeled as Ductile Moment-Resisting Frames, Dual System, Shear Wall System and Coupled Shear Wall system. The parameters studied have included variation of building heights, masses and stiffnesses and contact elastic spring stiffness. The effect of gap size on the pounding between adjacent reinforced concrete buildings during strong earthquakes has been also studied. The requirements for gap size of the Uniform Building Code (UBC-94), the Eurocode 8 (EC-8) and the Egyptian code for loads (ECL- 93) have been evaluated.

An artificial earthquake time history suitable for Egypt was generated and used to evaluate the requirements of ECL-93 for gap size. This artificial earthquake has a peak ground acceleration of 0.3g and a duration of 20 seconds and was chosen to represent the "probable design level" earthquakes for zone 3 in Egypt. This artificial earthquake can be considered "Moderate Earthquake" as required by the ECL-93.

The results of this study showed that pounding can be more severe load condition than the case of vibration without pounding. Therefore, neglecting the effects of possible pounding may lead to unsafe building design. The gap size of 2.0 cm required by the ECL-93 is seriously insufficient for the five examples analyzed in this study. Providing the gap size depending on the absolute sum (ABS) or the square root of sum of squares (SRSS) combinations of the peak displacements of the two buildings at the possible pounding level showed to be very conservative.

New simple empirical equations for calculating the minimum gap size between two adjacent buildings for the different seismic zones in Egypt have been proposed.

CHAPTER 1

CHAPTER 1

INTRODUCTION

1.1 GENERAL

In most cities of the world, specially in their centers, buildings are often constructed next to each other with no or small separation between them to minimize the waste of the high price land. When such closely spaced buildings have different dynamic characteristics as a result of a difference in height, mass, stiffness of structural elements and geometric properties, they will usually move out of phase during an earthquake. If the gap between such buildings is not adequate, they are likely to collide during an earthquake. Such collision is commonly referred to as "structural pounding". For each collision occurs, the buildings are subjected to short lateral impact forces not specifically accounted for in the conventional design process. These impact forces produce high amplitude, short-duration local accelerations, which could induce damage to structural members or mechanical and electrical components of the buildings. Furthermore, the overall dynamic response of the buildings can be amplified by earthquake pounding.

Many incidents of seismic pounding have been reported in past earthquakes [5,15,26,28,29,32]. Pounding of adjacent buildings has made damage worse, and may have caused total collapse of the buildings. The earthquake that struck Mexico City in 1985 has revealed the fact that pounding was present in more than 40% of 330 collapsed or severely damaged buildings surveyed, and in 15% of all cases it led to collapse [5]. This illustrates the significant seismic hazard of pounding by the large number of buildings damaged by pounding effect during a single earthquake. The 1989 Loma Prieta earthquake caused pounding between buildings over a wide geographical area including the cities of San Francisco, Oakland, Santa Cruz and Watsonville, where more than 200 cases of pounding were reported. Significant pounding was observed at sites over 90 Km from the epicenter thus indicating the possible catastrophic damage that may occur during future earthquakes having closer epicenters [26].