

Ain Shams University Faculty of Engineering Department of Structural Engineering

Nonlinear analysis of shear wall converted to braced frame under earthquake loading

A Thesis Submitted in partial fulfillment for the requirements of
The degree of
Master of Science in Civil Engineering

By

Ayman Osama Abd El Khalek B.Sc. (2012)

Graduate Student, Structural Engineering Department
Ain Shams University

Supervised by

Dr. Gamal Hussein Mahmoud

Associate Professor Structural Engineering Department Ain Shams University

Dr. Amgad Ahmed Talaat

Assistant Professor Structural Engineering Department Ain Shams University

Dr. Nasr Eid Nasr

Assistant Professor Structural Engineering Department Ain Shams University

Cairo 2018

EXAMINERS COMMITTEE

1- Prof. Dr. Mohamed M. Hussien Attabi Professor in Structural Engineering Department Faculty of Engineering Ain Shams University	()
2- Prof. Dr. Hala Mohamed El Kadi Professor at National Research Center	()
3- Dr. Gamal Hussien Mahmoud Associate Professor in Structural Engineering Faculty of Engineering Ain Shams University	()

Information about the Researcher

Name : Ayman Osama Abd El Khalek

Date of Birth : 23/3/1990

Place of Birth : Cairo – Egypt

Qualification : B.Sc. (Civil Engineering), Faculty of

Engineering, Ain Shams University, Cairo,

Egypt, June 2012

STATEMENT

This dissertation is submitted to Ain Shams University for the degree of

Master of Science in Civil Engineering (Structural).

The work included in this thesis was carried out by the author in the

Department of structural engineering, Faculty of engineer, Ain Shams University.

No part of this thesis has been submitted for a degree or qualification at other

university or institution.

Date : / /

Signature:

Name : Ayman Osama Abd El Kalek

i

ACKNOWLEDGMENT

First of all, thanks god to finish this research and I would like to thanks everyone who helps me. Many thanks for **Dr. Gamal Hussein,** Professor of theory of structures, Faculty of Engineering, Ain Shams University, for his great supervision, planning for this wok.

I am deeply indebted to **Dr. Nasr Eid,** Assistant Professor of theory of structures, Faculty of Engineering, Ain Shams University, for his kind supervision, support, guidance, help, encouragement and useful suggestions since the start of the wok.

I wish to extend my thanks to my family specially to my mother who always encourage me and direct me to the best way in my life Also, my wife who encourage me and care about me during work.

Finally, appreciation and deep gratitude to my brother, for his support and encouragement.

ABSTRACT

The objective of this research work is to study the shear wall converted to steel braced

frame under seismic loads. The behavior of shear walls and braced frames was

analyzed using nonlinear static analysis by nonlinear finite element analysis. A

parametric study was performed to assess the effect of building height, location of the

system, amount of reinforcement, and the fundamental time of the structure on the

structural elements capacity.

Shear walls converted to braced frame in top stories were studied by nonlinear static

analysis, which uses displacement based design method which depends on element

nonlinear behavior to calculate the effective stress and strain which effect on total

displacement of the building. The results are presented and show that shear walls can

be replaced with braced frame until reaching the buckling load for the steel member.

A comparison is created for story drift values with various structural codes.

Key Words: Pushover analysis; Nonlinear static analysis; Shear walls; Earthquake

loading; Capacity curve; Braced frame; Base shear; Displacement.

iii

TABLE OF CONTENTS

Statement	i
Acknowledgement	ii
Abstract	ii
Table of contents	iv
List of figures	V
List of tables	XV
Chapter (1): INTRODUCTION	1
1.1 General	1
1.2 Objective of the work	1
1.3 Outline of the thesis	3
Chapter (2): Literature review	4
2.1 Introduction	:
2.2 Braced frame system	:
2.2.1 Nonlinear analysis	
2.2.2 Linear analysis	2
2.3 Shear wall system	4
2.3.1 Modeling of shear wall	4
2.3.2 Pushover analysis for shear wall	4
2.4 Summary	5
Chapter (3): Methodology	6
3.1 Introduction	ϵ
3.2 Pushover static load	6
3.3 Hysteresis Models	6
3.4 Hinge length	6
3.5 Fiber P-M2-M3 hinge	6
3.6 Analysis modeling	ϵ
3.6.1 Lateral load distribution	ϵ
3.7 Concrete shear walls	6
3.8 Steel braced frames	6
3.9 P-delta effect	6
3.10 Steps of nonlinear analysis for shear wall converted to braced frame using pushover static procedure	ϵ
Chapter (4): Verification of pushover analysis method	7
4.1 Structural system model	7
4.2 Load criteria	7
4.3 Analytical results	7

5.1 Structural model. 77 5.2 Analysis method. 78 5.3 Results to get demand / capacity ratio. 81 Chapter (6): Parametric study 83 6.1 Description of parameters 83 6.2 Analytical results 86 6.2 Discussion of results 161 Chapter (7): Conclusions and future work 163 7.1 Conclusions 163 7.2 Design recommendations 164 7.3 Suggested researches for future work 164 References 165	Chapter (5): Analytical modeling using ETABS	77
5.3 Results to get demand / capacity ratio. 81 Chapter (6): Parametric study. 83 6.1 Description of parameters. 83 6.2 Analytical results. 86 6.2 Discussion of results. 161 Chapter (7): Conclusions and future work. 163 7.1 Conclusions. 163 7.2 Design recommendations. 164 7.3 Suggested researches for future work. 164	5.1 Structural model	77
Chapter (6): Parametric study 83 6.1 Description of parameters 83 6.2 Analytical results 86 6.2 Discussion of results 161 Chapter (7): Conclusions and future work 163 7.1 Conclusions 163 7.2 Design recommendations 164 7.3 Suggested researches for future work 164	5.2 Analysis method	78
6.1 Description of parameters836.2 Analytical results866.2 Discussion of results161Chapter (7): Conclusions and future work1637.1 Conclusions1637.2 Design recommendations1647.3 Suggested researches for future work164	5.3 Results to get demand / capacity ratio	81
6.2 Analytical results 86 6.2 Discussion of results 161 Chapter (7): Conclusions and future work 163 7.1 Conclusions 163 7.2 Design recommendations 164 7.3 Suggested researches for future work 164	Chapter (6): Parametric study	83
6.2 Discussion of results	6.1 Description of parameters	83
Chapter (7): Conclusions and future work.1637.1 Conclusions1637.2 Design recommendations1647.3 Suggested researches for future work164	6.2 Analytical results	86
7.1 Conclusions1637.2 Design recommendations1647.3 Suggested researches for future work164	6.2 Discussion of results	161
7.2 Design recommendations.1647.3 Suggested researches for future work.164	Chapter (7): Conclusions and future work	163
7.3 Suggested researches for future work	7.1 Conclusions	163
	7.2 Design recommendations	164
References 165	7.3 Suggested researches for future work	164
	References	165

LIST OF FIGURES

Figure (1-1): Types of bracing and facilities needed through it	2
Figure (1-2): Opening in concrete shear wall and the solution	3
Figure (2-1): Lateral load resisting system	5
Figure (2-2): The loading considered for the two model systems	6
Figure (2-3): Detailing of the moment RC frame (F1) and the braced RC fram	nes (FX1
and FX2)	6
Figure (2-4): Test setup and measurement Instrumentations	7
Figure (2-5): Lateral load-drift hysteresis of frame F1	8
Figure (2-6): Braced frame FX1 after the test	8
Figure (2-7): Lateral load-drift hysteresis of frame FX1	9
Figure (2-8): Lateral load-drift hysteresis of frame FX2	9
Figure (2-9): Degradation of the lateral stiffness of test frames	10
Figure (2-10): Variation of energy dissipation with the applied displacement.	10
Figure (2-11): Comparison between the numerical lateral load-drift pushove	er curves
of the moment frame, $F1$, the braced RC frame, $FX1$	11
Figure (2-12): Comparison between the numerical lateral load-drift pushove	er curves
of the moment frame, F1, the braced RC frame, FX2	11
Figure (2-13): Structural models	12
Figure (2-14): Reinforcement details.	12
Figure (2-15): Bracing models.	12
Figure (2-16): Pushover curve for Tube 140 (6 stories)	13
Figure (2-17): Pushover curve for Tube 140 (3 stories)	13
Figure (2-18) Pushover curve for Tube 180 (6 stories)	13
Figure (2-19): Pushover curve for Tube 180 (3 stories)	13
Figure (2-20): Pushover curve for Tube 240 (6 stories)	13
Figure (2-21): Pushover curve for Tube 240 (3 stories)	13
Figure (2-22): Lateral drift for Tube 180 (3 stories)	14
Figure (2-23): Lateral drift for Tube 180 (6 stories)	14
Figure (2-24): Ductility and different types of Tube sections	14
Figure (2-25): Influence of Tube sections	15
Figure (2-26): Influence of HEA sections	15
Figure (2-27): Influence of IPE sections.	16

Figure (2-28): Bare frame details (F1)
Figure (2-29): Steel braced frame details (F3)
Figure (2-30) : Test setup
Figure (2-31): Mode of failure of (F1)
Figure (2-32): Mode of failure of (F3)
Figure (2-33): Load-displacement hysteresis curve of specimen F1 (bare frame)
and F3 (steel braced frame)19
Figure (2-34): Determination of yield and failure displacement20
Figure (2-35): Stiffness degradation of the tested specimen21
Figure (2-36): Building Plan
Figure (2-37): Building elevation
Figure (2-38): Elevation of cross braced structure
Figure (2-39): Elevation of Chevron braced structure
Figure (2-40) : Elevation of Diagonal braced structure
Figure (2-41) : Elevation of K-braced structure
Figure (2-42): Plan representing the position of bracing
Figure (2-43): 3D model of X-braced sample frame, A; 3x1 braced, B: corner
braced
Figure (2-44): Max. Lateral displacement of (4 story 3x1 braced)33
Figure (2-45): Max. Lateral displacement of (4 story) corner braced
Figure (2-46): Max. Lateral displacement of only braced frame of (4story) corner
braced
Figure (2-47): Story drift for (4 story) in Y-direction
Figure (2-48): Max. Lateral Displacements of (6story) 3x1 braced35
Figure (2-49): Max. Lateral Displacements of (6story) corner braced35
Figure (2-50): Max. Lateral Displacements of braced frame of (6story) corner
braced
Figure (2-51): Story drift of (6story)
Figure (2-52): lateral displacement graph of (9story) 3x1 braced
Figure (2-53): Graph of lateral displacement of (9story) corner braced37
Figure (2-54): graph of lateral displacement of braced frame for(9story) corner
braced
Figure (2-55): Story drift graph of (9story)
Figure (2-56): Lateral displacement of (13story) (3x1braced)

Figure (2-57): Lateral displacement of (13story) (corner braced)	39
Figure (2-58) Lateral displacement of braced frame for (13story) (corner braced).	40
Figure (2-59) Story drift of (13story)	40
Figure (2-60): Different finite elements models for shear wall	41
Figure (2-61) Multi-layer shell element model	42
Figure (2-62): P-M-M curves.	42
Figure (2-63): Shear wall modeling as multilayer shell and mid pier frame	42
Figure (2-64): Reinforcement distribution for shear wall and column	43
Figure (2-65) Deformation for multi-layer shell and mid pier frame element	43
Figure (2-66) Number of plastic hinges for different 3 stories	44
Figure (2-70): Pushover curves for 3 stories.	44
Figure (2-71) Pushover curves for 5 stories.	45
Figure (2-72) Pushover curves for 7 stories.	45
Figure (2-73): Drift values for 3 stories.	46
Figure (2-74) : Drift values for 5 stories.	46
Figure (2-75): Drift values for 7 stories.	47
Figure (2-76): Building plan with shear wall location.	47
Figure (2-77): Directions of pushover load cases.	48
Figure (2-78): Building model.	48
Figure (2-79) : Design response spectrum.	49
Figure (2-80): Modeling of shear wall.	49
Figure (2-81): Pushover analysis curve with performance point location	50
Figure (2-82): Model configurations for boundary element study	51
Figure (2-83): Model configurations for opening study	51
Figure (2-84): Model configurations for slenderness study	52
Figure (2-85): Pushover curves for boundary element models	53
Figure (2-86): Pushover curves for opening models with boundary elements	53
Figure (2-87): Pushover curves for opening models without boundary elements	54
Figure (2-88): Pushover curves for slenderness models	54
Figure (2-89): Model one frame system without shear wall	55
Figure (2-90): Frame system without shear wall at position 1	55
Figure (2-91): Frame system without shear wall at position 2	55
Figure (2-92): Frame system without shear wall at position 3	55
Figure (2-93): Displacement values for model 1	56

Figure (2-94): Displacement values for model 25
Figure (2-95): Displacement values for model 357
Figure (2-96) Displacement values for model 4
Figure (3-1): Idealized properties for analysis models
Figure (3-2): Hysteretic force-deformation behavior from tests
Figure (3-3): Backbone representation of hysteretic behavior
Figure (3-4): Idealized properties for analysis models
Figure (3-5): Fiber discretization in a reinforced concrete section63
Figure (3-6): Vertical distribution of applied base shear
Figure (3-7): Plastic Hinge Rotation in Shear Wall where Flexure Dominate
Inelastic Response
Figure (3-8): Story Drift in Shear Wall where Shear Dominates Inelastic
Response
Figure (3-9): Concentrically braced frame67
Figure (3-10): P-delta about column
Figure (3-11): Response of structural building under earthquake loading
with and without P-delta effects69
Figure (3-12): Steps for pushover procedure in ETABS program71
Figure (4-1): Lateral load resisting system
Figure (4-2): The loading considered for the model system
Figure (4-3): Detailing of the braced RC frames
Figure (4-4): Schematic of the test setup and measurement instrumentation74
Figure (4-5): Gravity load assigned to the system
Figure (4-6): Earthquake loading applied by pushover analysis
Figure (4-7): Pushover curve with dropped load for braced frame system due to
buckling of steel bracing
Figure (5-1): Building Plan
Figure (5-2) : ETABS 3D model structure
Figure (5-3) : Stress-strain curve for reinforced concrete material
Figure (5-4) : Stress-strain curve for steel reinforcement material80
Figure (5-5) : Stress-strain curve for steel section material
Figure (5-6) : Base Shear-Displacement curve in X-direction82
Figure (5-7) : Base Shear-Displacement curve in Y-direction82
Figure (6-1) : General structural plan83

Figure (6-2)	: Structural plan with Eccentricity in Y-direction	83
Figure (6-3)	: Structural plan with Eccentricity in X-direction	83
Figure (6-4)	: Structural elements of RC shear wall converted to steel brace	ed RC
frame		84
Figure (6-5)	: Base Shear-Displacement curve for Model 1	88
Figure (6-6)	: Base Shear-Displacement curve for Model 2	88
Figure (6-7)	: Base Shear-Displacement curve for Model 3	89
Figure (6-8)	: Base Shear-Displacement curve for Model 4	89
Figure (6-9)	: Base Shear-Displacement curve for Model 5	90
Figure (6-10)	: Base Shear-Displacement curve for Model 6	90
Figure (6-11)	: Base Shear-Displacement curve for Model 7	91
Figure (6-12)	: Base Shear-Displacement curve for Model 8	91
Figure (6-13)	: Base Shear-Displacement curve for Model 9	92
Figure (6-14)	: Base Shear-Displacement curve for Model 10	92
Figure (6-15)	: Base Shear-Displacement curve for Model 11	93
Figure (6-16)	: Base Shear-Displacement curve for Model 12	93
Figure (6-17)	: Base Shear-Displacement curve for Model 13	94
Figure (6-18)	: Base Shear-Displacement curve for Model 14	94
Figure (6-19)	: Base Shear-Displacement curve for Model 15	95
Figure (6-20)	: Base Shear-Displacement curve for Model 16	95
Figure (6-21)	: Base Shear-Displacement curve for Model 17	96
Figure (6-22)	: Base Shear-Displacement curve for Model 18	96
Figure (6-23)	: Base Shear-Displacement curve for Model 19	97
Figure (6-24)	: Base Shear-Displacement curve for Model 20	97
Figure (6-25)	: Base Shear-Displacement curve for Model 21	98
Figure (6-26)	: Base Shear-Displacement curve for Model 22	98
Figure (6-27)	: Base Shear-Displacement curve for Model 23	99
Figure (6-28)	: Base Shear-Displacement curve for Model 24	99
Figure (6-29)	: Base Shear-Displacement curve for Model 25	100
Figure (6-30)	: Base Shear-Displacement curve for Model 26	100
Figure (6-31)	: Base Shear-Displacement curve for Model 27	101
Figure (6-32)	: Base Shear-Displacement curve for Model 28	101
Figure (6-33)	: Base Shear-Displacement curve for Model 29	102
Figure (6-34)	: Base Shear-Displacement curve for Model 30	102

Figure (6-35) : Base Shear-Displacement curve for Model 31
Figure (6-36) : Base Shear-Displacement curve for Model 32
Figure (6-37) : Base Shear-Displacement curve for Model 33
Figure (6-38) : Base Shear-Displacement curve for Model 34
Figure (6-39) : Base Shear-Displacement curve for Model 35
Figure (6-40) : Base Shear-Displacement curve for Model 36
Figure (6-41) : Base Shear-Displacement curve for Model 37
Figure (6-42) : Base Shear-Displacement curve for Model 38
Figure (6-43) : Base Shear-Displacement curve for Model 39
Figure (6-44) : Base Shear-Displacement curve for Model 40
Figure (6-45) : Base Shear-Displacement curve for Model 41
Figure (6-46) : Base Shear-Displacement curve for Model 42
Figure (6-47) : Base Shear-Displacement curve for Model 43
Figure (6-48) : Base Shear-Displacement curve for Model 44
Figure (6-49) : Base Shear-Displacement curve for Model 45
Figure (6-50) : Base Shear-Displacement curve for Model 46
Figure (6-51) : Base Shear-Displacement curve for Model 47
Figure (6-52) : Base Shear-Displacement curve for Model 48
Figure (6-53) : Relation between stress ratio and percentage of braced frame height
to total building height for 10 stories in X-direction
Figure (54-6): Relation between stress ratio and percentage of braced frame height to
total building height for 15 stories in X-direction
Figure (55-6): Relation between stress ratio and percentage of braced frame height to
total building height for 20 stories in X-direction
Figure (56-6): Relation between stress ratio and percentage of braced frame height
to total building height for 25 stories in X-direction
Figure (57-6): Relation between stress ratio and percentage of braced frame height to
total building height for 30 stories in X-direction
Figure (58-6): Relation between stiffness and story level for buildings (10, 15, 20,25
& 30) in X -direction
Figure (6-59): Relation between Time period and percentage of braced frame height
to total building height for (10 stories in X-direction)
Figure (6-60) : Relation between Time period and percentage of braced frame height
to total building height for (15 stories in X-direction)

Figure (61-6): Relation between Time period and percentage of braced frame height
to total building height for (20 stories in X-direction)
Figure (62-6): Relation between Time period and percentage of braced frame height
to total building height for (25 stories in X-direction)
Figure (63-6): Relation between Time period and percentage of braced frame height
to total building height for (30 stories in X-direction)
Figure (64-6): Relation between number of braced frame stories from top and Drift
value in X direction
Figure (65-6): Relation between Thickness and stress ratio of the building elements
for 20 stories with one braced frame in X direction
Figure (66-6): Relation between Thickness and stress ratio of the building elements
for 20 stories with two braced frame in X direction
Figure (67-6): Relation between Thickness and stress ratio of the building elements
for 20 stories with three braced frame in X direction120
Figure (68-6): Relation between Thickness and stress ratio of the building elements
for 20 stories with four braced frame in X direction
Figure (69-6): Relation between Thickness of shear wall and Drift value for 20
stories in X direction
Figure (70-6): Relation between Thickness of shear wall and Drift value for 20
stories in X direction
Figure (71-6): Relation between Thickness of shear wall and Drift value for 20
stories in X direction
Figure (6-72): Building plan for 20 stories with eccentricity in Y direction123
Figure (6-74): Base Shear-Displacement curve for Model 1
Figure (6-75) : Base Shear-Displacement curve for Model 2
Figure (6-76) : Base Shear-Displacement curve for Model 3
Figure (6-77): Base Shear-Displacement curve for Model 4
Figure (6-78): Base Shear-Displacement curve for Model 5
Figure (6-79): Base Shear-Displacement curve for Model 6
Figure (6-80) : Base Shear-Displacement curve for Model 7
Figure (6-81): Base Shear-Displacement curve for Model 8
Figure (6-82): Base Shear-Displacement curve for Model 9
Figure (6-83): Base Shear-Displacement curve for Model 10
Figure (6-84) · Base Shear-Displacement curve for Model 11