



## EXPERIMENTAL INVESTIGATION OF STRENGTHENING SLAB-COLUMN CONNECTIONS WITH CFRP FAN

By

### Eman Abd Al Ghaffar Ahmed Essa

A Thesis Submitted to the
Faculty of Engineering at Cairo University
in Partial Fulfillment of the
Requirements for the Degree of
MASTER OF SCIENCE
in
STRUCTURAL ENGINEERING

FACULTY OF ENGINEERING, CAIRO UNIVERSITY
GIZA, EGYPT
2018

## EXPERIMENTAL INVESTIGATION OF STRENGTHENING SLAB-COLUMN CONNECTIONS WITH CFRP FAN

By

### Eman Abd Al Ghaffar Ahmed Essa

A Thesis Submitted to the
Faculty of Engineering at Cairo University
in Partial Fulfillment of the
Requirements for the Degree of
MASTER OF SCIENCE
in
STRUCTURAL ENGINEERING

Under the Supervision of

#### Prof. Dr. Hamed Mohamed Mahmoud Hadhoud

Dr. Khaled Farouk Omar El- Kashif

Professor of Concrete Structures Structural Engineering Department Faculty of Engineering, Cairo University Assistant Professor of Concrete Structures Structural Engineering Department Faculty of Engineering, Cairo University

FACULTY OF ENGINEERING, CAIRO UNIVERSITY
GIZA, EGYPT
2018

## EXPERIMENTAL INVESTIGATION OF STRENGTHENING SLAB-COLUMN CONNECTIONS WITH CFRP FAN

By

### **Eman Abd Al Ghaffar Ahmed Essa**

A Thesis Submitted to the
Faculty of Engineering at Cairo University
in Partial Fulfillment of the
Requirements for the Degree of
MASTER OF SCIENCE
in
STRUCTURAL ENGINEERING

Approved by the Examining Committee	
Prof. Dr. Hamed Mohamed Mahmoud Salem Hadhoud	Thesis Main Advisor
Prof. Dr. Wael Mohamed Al Degwy	Internal Examiner
Prof. Dr. Hatem Hamdy Ghaith	External Examiner

FACULTY OF ENGINEERING, CAIRO UNIVERSITY
GIZA, EGYPT
2018

Engineer's Name: Eman Abdalghaffar Ahmed Essa

**Date of Birth:** 1/8/1990 **Nationality:** Egyptian

**E-mail:** eng\_eman\_abdalghaffar@hotmail.com

**Phone:** (+20) 01117442007

Address: 541-saudi building – hadayek al koba

Registration Date: 01/10/2013
Awarding Date: ..../2018
Degree: Master of Science
Department: Structural Engineering



#### **Supervisors:**

Prof. Dr. Hamed Mohamed Mahmoud Hadhoud

Dr. Khaled Farouk Omar El-Kashif

#### **Examiners:**

Prof. Dr. Hamed Mohamed Mahmoud Salem Hadhoud (Thesis Main Advisor)
Prof. Dr. Wael Mohamed Al Degwy (Internal Examiner)
Prof. Dr. Hatem Hamdy Ghaith (External Examiner)

-Professor of Concrete Structures Housing and Building Research Center

#### Title of Thesis:

"Experimental Investigation of Strengthening Slab-Column Connections With CFRP Fan ".

#### **Key Words:**

Punching Shear Strengthening, Flat slabs, RC, CFRP string.

#### **Summary**:

The current thesis presents an experimental investigation of strengthening flat slab column connections in punching shear using CFRP string. Two types of strengthening were used; strengthening with CFRP string (fan shape) and strengthening with steel bolts. Parameters which were studied were the number of CFRP fan strengthener, arrangement or configuration of CFRP fan strengthener, and the type of strengthener (CFRP string or steel bolts). All the specimens were strengthened before starting loading, the results of the tested specimens showed that CFRP fan strengthening technique is able to enhance both the punching shear capacity and the ductility of tested flat slab specimens.

### **ACKNOWLEDGMENT**

I would like to express my gratitude to my parents, my brothers, my friends, especially my mother, my brother Salah Abdalghaffar who always support me and stand beside me in every steps in my life.

I would like to gratefully acknowledge the significant assistance and guidance of my advisors during my work on this thesis. I would also like to thank all the people who have helped me to accomplish my thesis.

I wish to express my sincere appreciation to my major advisor, Prof. Dr. Hamed Mohamed Hadhoud for his academic support through the entire study period and his advices to finish this work as it should be.

Special thanks to Dr. Khaled Farouk Omar El-Kashif for his technical support, providing valuable material and providing the required needs to finish this work as it should be.

# **DEDICATION**

Dedicated to my parents, my brothers with love

# TABLE OF CONTENTS

ACKNOWLEDGMENT	I
DEDICATION	II
TABLE OF CONTENTS	III
LIST OF TABELS	VII
LIST OF FIGURES	IX
NOMENCLATURE	
ABSTRACT	
ADSTRACT	III V
CHAPTER ONE: INTRODUCTION	
1-1 General	
1-2 Objective and Scope of Work	
1-3 Thesis Outline	2
CHAPTER TWO: LITERATURE REVIEW2-1 General	
2-2 Punching Shear Strength of Flat Slabs	5
2-2-1 Critical Sections [1]	5
2-2-2 Design Codes.	7
2-2-2-1 The Egyptian Code of Practice [2]	
2-2-2-2 JSCE Guideline for Concrete No.15[3]	
2-2-2-3 American Concrete Institute (ACI) [4]	
2-3 Previous Research Works	
2-3-1 Strengthening with Steel Bolts and Steel Plates	
2-3-2 Strengthening with Fiber Reinforced Polymer Composites	
2-4 Fiber Reinforced Polymer [18]	
2-4-1 Historical Background	
2-5 Fiber Reinforced Polymer Composition	
2-5-1 Resins	
2-5-1-1 Primer	
2-5-1-2 Putty Fillers	
2-5-1-3 Saturating Resin.	
2-5-1-4 Adhesive	
2-5-2 Fibers	25

2-5-2-1 Filament	25
2-5-2-2 Yarn	26
2-5-2-3 Tow	26
2-5-2-4 Roving	26
2-5-2-5 Chopped Strands	27
2-5-2-6 Fiber Mats	28
2-5-2-7 Fabrics	28
2-5-2-8 Unidirectional Fabrics	29
2-5-3 Protective Coating	30
2-6 Physical Properties	30
2-6-1 Density	30
2-6-2 Coefficient of Thermal Expansion.	30
2-6-2-1 Effect of High Temperature Beyond the Tg	31
2-7 Mechanical Properties	
2-7-1 Tensile Strength	32
2-7-2 Compressive Behavior of Externally Bonded FRP	32
2-7-3 Creep Rupture	32
2-7-4 Fatigue	33
CHAPTER THREE: EXPERIMENTAL WORK	34
	2.4
3-1 Introduction	
3-2 Material Specifications	
3-2-1 Coarse Aggregate.	
3-2-2 Fine Aggregate	
3-2-3 Cement	
3-2-4 Steel	
3-2-5-1 SikaWrap FX-50C	
3-2-5-2 Sikadur-330	
3-2-5-2 Sikadur-530	
3-2-5-4 Kemapoxy 165	
3-3 Test Specimens.	
3-4 Mixing, Placing and Curing Method of Specimens	
3-4-1 Concrete Mix Design	
3-4-2 Concrete Mixing, Placing and Curing	
3-5 Test Setup	
3-6 Measuring Instruments	
3-6-1 Concrete Deflection.	
3-6-2 Steel Bars Strain.	
3-6-3 Vertical Loads.	
3-6-4 Cracking.	
3-7 Strengthening Procedure.	

3-7-1 Using CFRP Fan.	47
3-7-2 Strengthening Using Shear Studs or Steel Bolts	
3-8 Configuration and Number of Shear Connectors	
CHAPTER FOUR: EXPERIMENTAL RESULTS	55
4-1 Introduction	
4-2 Test Results of the Specimens.	
4-2-1 Specimen S1-R-00	
4-2-1-1 Cracking Pattern and Mode of Failure	
4-2-1-2 Load Deflection Curve	
4-2-1-3 Strains in Reinforcing Bars	
4-2-2 Specimen S2-SC-8S	
4-2-2-1 Cracking Pattern and Mode of Failure	
4-2-2-2 Load Deflection Curve	
4-2-2-3 Strains in Reinforcing Bars	59
4-2-3 Specimen S3-SC-8D	
4-2-3-1 Cracking Pattern and Mode of Failure	59
4-2-3-2 Load Deflection Curve	60
4-2-3-3 Strains in Reinforcing Bars	61
4-2-4 Specimen S4-SS-8S	62
4-2-4-1 Cracking Pattern and Mode of Failure	62
4-2-4-2 Load Deflection Curve	
4-2-4-3 Strains in Reinforcing Bars	
4-2-5 Specimen S5-SC-16S	64
4-2-5-1 Cracking Pattern and Mode of Failure.	
4-2-5-2 Load Deflection Curve.	66
4-2-5-3 Strains in Reinforcing Bars	
4-2-6 Specimen S6-SC-16D	67
4-2-6-1 Cracking Pattern and Mode of Failure	
4-2-6-2 Load Deflection Curve.	
4-2-6-3 Strains in Reinforcing Bars	69
CHAPTER FIVE: ANALYSIS OF RESULTS	70
5-1 Introduction	70
5-2 Group A, Effect of Using Different Number and Arrangement of CFRP string	70
5-2-1 Discussion of Test Results for the Specimens S1-R-00, S2-SC-8S and S5-SC-10	6S.71
5-2-1-1 Cracking Behavior of Tested Specimens	71
5-2-1-2 Load- Deflection Behavior	73
5-2-1-3 Ductility of Slabs S1-R-00, S2-SC-8S and S5-SC-16S	75
5-2-1-4 Stiffness of Slabs S1-R-00, S2-SC-8S and S5-S-8S	76
5-3 Group B, Effect of Using Double Area of CFRP String	79
5-3-1 Discussion of Test Results for the Specimens S1-R-00, S3-SC-8D and S6-SC-	
16D	79

5-3-1-1 Cracking Behavior of Tested Specimens	79
5-3-1-2 Load- Deflection Behavior for Specimens	81
5-3-1-3 Ductility of Specimens S3-SC-8D and S6-SC-16D	83
5-3-1-4 Stiffness of Slab S3-SC-8D and S6-SC-16D	84
5-4 Group C, Effect of Strengthening Using Steel Bolts	86
5-4-1 Discussion of Test Results for the Specimen S4-SS-8S	86
5-4-1-1 Cracking Behavior of Tested Specimens	
5-4-1-2 Load- Deflection Behavior for Specimen S4-SS-8S	87
5-4-1-3 Ductility of Slab S4-SS-8S	
5-4-1-4 Stiffness of Slab S4-SS-8S	89
5-5 Comparison of Results of the Current Research Tested Specimens' with the Previou	s Experimental
Program Specimens'	92
5-5-1 Comparison of Results of Specimen S4-SS-8S (slab strengthened with 8 steel be	olts) with
Previous Research Concerned with using Steel Bolts	92
5-5-2 Comparison the Results of Specimens Strengthened with Single Area of G	CFRP fan with
the Previous Experimental Program used the Same Strengthening Technique	93
5-5-2-1 Details of Tested Specimens at Each Experimental Program	93
5-5-2-2 Punching Shear Failure Load of Each Experimental Program	95
5-5-2-3 Ductility of Tested Specimens at Each Experimental Program	97
5-6 Strengthening Materials Cost Influence	98
5.6.1 Strengthening with CFRP String	98
5.6.2 Strengthening with Steel Bolts	98
5.6.3 Comparing the Methods of Strengthening	99
5-7 The Expected Punching Shear Capacity of Tested Specimens According to D	ifferent
Codes	102
5-7-1 Expected Punching Shear Capacity According to ECP[2]	102
5-7-2 Expected Punching Shear Capacity According to JSCE[3]	103
5-7-3 Expected Punching Shear Capacity According to ACI [4]	104
5-7-4 Calculation the Expected Punching Shear Capacity of Specimen (S4-SS-8	SS) Specimen
Strengthened with 8 Steel Bolts using ACI code (ACI 421.1R-08) [19]	106
CHAPTER SIX: CONCLUSIONS AND RECOMMENDATIONS	<b>S</b> 107
6-1 Conclusions	107
6-2 Recommendations	108
REFERENCES	100

# LIST OF TABLES

Table 2-1: Typical coefficient of thermal expansion for FRP material [18]	31
Table 3-1: Mechanical properties of steel.	35
Table 3-2: Properties of SikaWrap FX-50C	36
Table 3-3: Properties of Sikadur 330	36
Table 3-4: Properties of Sikadur-52	37
Table 3-5: Properties of Kemapoxy 165	37
Table 3-6: The characterises of the tested specimens	38
Table 3-7: Components of specimens mix design (per $m^3$ )	39
Table 3-8: Cubes test results show the compressive strength values after 7 days of casting	42
Table 3-9: Cubes test results show the compressive strength values of each specimen that determined at experiment day	43
Table 4-1: Details of strengthened slabs	55
Table 5-1: Properties of specimens in group (A)	70
Table 5-2: Ductility factor of specimens S1-R-00, S2-SC-8S and S5-SC-16S	75
Table 5-3: Ductility of slabs S1-R-00, S2-SC-8S and S5-SC-16S using area under curve	76
Table 5-4: Stiffness degradation of specimens S1-R-00, S2-SC-8S and S5-SC-16S	77
Table 5-5: Properties of specimens in group (B)	79
Table 5-6: Ductility factor of specimens S3-SC-8D and S6-SC-16D	83
Table 5-7: Ductility of slabs S3-SC-8D and S6-SC-16D	83
Table 5-8: Stiffness degradation of specimens S3-SC-8D and S6-SC-16D	84
Table 5-9: Ductility factor of specimen S4-SS-8S	89
Table 5-10: Ductility of slab S4-SS-8S.	89
Table 5-11: Stiffness degradation of specimen S4-SS-8S	90
Table 5-20: Specification of the examined specimens	92
Table 5-21: Details of tested specimens.	93

Table 5-22: Yielded load and ultimate failure load of specimens strengthened with FRP	
Table 5-23: Yielded load and ultimate failure load of specimens strengthened with CFR	P fan
Table 5-24: Ductility factor of specimens strengthened with FRP fan	
Table 5-25: Values of ductility using the two methods for specimens strengthened w fan	
Table 5-26: Material cost for strengthening of one hole with single area of CFRP String	;98
Table 5-27: Material cost for specimens strengthened with CFRP string	98
Table 5-28: Material cost for specimen S4-SS-8S strengthened with steel bolts	98
Table 5-29: Percentage of increased ductility and the cost of strengthening of each slab.	100
Table 5-30: Percentage of increased capacity and the cost of strengthening of each slab.	100
Table 5-31: The summary of punching shear capacities calculation of tested specimens to ECP.	_
Table 5-32: The summary of punching shear capacities calculation of tested specimens to JGCE at (d/2) from the outermost peripheral line of CFRP string	•
Table 5-33: The summary of punching shear capacities calculation of tested specimens to ACI 318 at $(d/2)$ from the outermost peripheral line of CFRP string	
Table 5-34: Summary of the increasing ratios in punching shear failure load acl experimental testes compared with the expected punching shear capacity calculated using codes	g different

# LIST OF FIGURES

Figure 2.1 Telekom Tower, Kuala Lumpur, Malaysia, 310 m (1,017 ft) tall, has 77 floors Constructed with flat slab system in 2001
Figure 2.2 Piper's Row Car Park, Wolverhampton, UK, 1997(built in 1965)4
Figure 2.3 Critical shear perimeter of internal, external and corner column [1]5
Figure 2.4 Critical shear perimeter a) without drop panel b) with column head [1]6
Figure 2.5 Loaded area and critical shear perimeter of interior column [1]6
Figure 2.6. Influence of loaded area on punching shear capacity [3]8
Figure 2.7 The design cross section with different column shape [3]9
Figure 2.8 Distribution of nominal shear forces along the control perimeter at 0.5d from the column face for rectangular and square column [6]
Figure 2.9 The two configurations of prestressed bolts arrangement in the tested slabs [20]
Figure 2.10 The arrangement of steel bolts at tested specimens [7]12
Figure 2.11 Different anchorage technique approaches [8]
Figure 2.12 Post installed shear reinforcement: (a) typical cross section; (b)view of nut, washers and bar; and (c) detail of anchor head [9]
Figure 2.13 Plates and stud's arrangement in the tested specimens [10]15
Figure 2.14 FRP rods and screw bolts arrangement on slab around column [11]16
Figure 2.15 Different strengthening systems: a) case of steel links; b) case of FRP stirrups [12]
Figure 2.16 CFRP strengthening schemes [13]
Figure 2.17 Frame used for plate pre-stressing [14]
Figure 2.18: Applied strengthening types & reinforcement details of concrete slabs [15]20
Figure 2.19 Strengthening technique using FRP fan [16]
Figure 2.20 FRP anchor layout [17]
Figure 2.21 Shape and manufacture filament
Figure 2.22 Shape of assembled roving and direct roving
Figure 2.23 Shape of fiber glass copped strand27

Figure 2.24 Shape of fiber glass mats2
Figure 2.25 Shape of unidirectional carbon fabrics
Figure (2.26-a) The different types of fabrics
Figure (2.26-b) Shape of Hybrid fabric (twill weave) of aramid and carbon
Figure 3.1 Coarse aggregate utilized in the Mix
Figure 3.2 Fine aggregate utilized in the Mix
Figure 3.3 Concrete dimension and reinforcement details of slab-interior column connection
3
Figure 3.4 The wooden form coated with oil, and arrangement of the steel mesh4
Figure 3.5 PVC pipes used to keep the holes positions
Figure 3.6 The operation of casting concrete at the wooden form4
Figure 3.7 The mechanical vibrato used during casting
Figure 3.8 The surface of casted specimens after it has been leveled
Figure 3.9 Test set up for the examined specimen4
Figure 3.10 Locations of LVDTS4
Figure 3.11 Where the electrical strain gauge installed from the column face
Figure 3.12 The steps of installation the electrical strain gauge
Figure 3.13 The setting of hydraulic jack, load cell with the specimens
Figure 3.14 Preparation steps of the slabs for strengthening with CFRP fan48
Figure 3.15 Installation steps of CFRP fan
Figure 3.16 Installation steps of steel bolts
Figure 3.17 Slab S2-SC-8S strengthened with eight CFRP string
Figure 3.18 Slab S3-SC-8D strengthened with 16 CFRP string double area of carbon fiber string
Figure 3.19 Slab S4-SS-8S strengthened with eight steel bolts
Figure 3.20 Slab S5-SC-16S strengthened with 16 CFRP string single area of carbon fiber string
Figure 3.21 Slab S6-SC-16D strengthened with 16 CFRP string double area of carbon fiber string

Figure 4.1 Cracking pattern for specimen S1-R-00 from tension side
Figure 4.2 Load central deflection for specimen S1-R-00
Figure 4.3 Vertical & Horizontal steel strain for specimen S1-R-00 near the column area57
Figure 4.4 Cracking pattern for specimen S2-SC-8S from tension side
Figure 4.5 Load central deflection for specimen S2-SC-8S
Figure 4.6 Vertical & Horizontal steel strain for specimen S2-SC-8S near the column area59
Figure 4.7 Cracking pattern for specimen S3-SC-8D from tension side
Figure 4.8 Load central deflection for specimen S3-SC-8D
Figure 4.9 Vertical & Horizontal steel strain for specimen S3-SC-8D near the column area
Figure 4.10 Cracking pattern for specimen S4-SS-8S from tension side
Figure 4.11 Load central deflection for specimen S4-SS-8S
Figure 4.12 Vertical & Horizontal steel strain for specimen S4-SS-8S near the column area
Figure 4.13 Showed splitting of CFRP sting at failure65
Figure 4.14 Cracking pattern for specimen S5-SC-16S from tension side65
Figure 4.15 Load central deflection for specimen S5-SC-16S
Figure 4.16 Vertical & Horizontal steel strain for specimen S5-SC-16S near the column area
Figure 4.17 Cracking pattern for specimen S6-SC-16D from tension side
Figure 4.18 Load central deflection for specimen S6-SC-16D
Figure 4.19 Vertical & Horizontal steel strain for specimen S6-SC-16D near the column area
Figure 5.1 Cracking pattern for specimen S1-R-0071
Figure 5.2 Cracking pattern of specimen S2-SC-8S
Figure 5.3 Cracking pattern of specimen S5-SC-16S
Figure 5.4 Showed splitting of CFRP string at failure73
Figure 5.5 Load deflection curve of specimens S1-R-00, S2-SC-8S and S5-SC-16S74
Figure 5.6 Load deflection curve of specimens S1-R-00, S2-SC-8S and S5-SC-16S75
Figure 5.7 Definition of yield deflection