

AIN SHAMS UNIVERSITY FACULTY OF ENGINEERING STRUCTURAL DEPARTMENT

SHEAR STRENGTHENING OF REINFORCED CONCRETE T-BEAMS USING ANCHORED FRP WRAPS

BY

Nouran Mohamed Mahmod Taha

B.Sc. 2012 STRUCTURAL DIVISION CIVIL ENGINEERING DEPARTMENT 6 OCTOBER UNIVERSITY

Prof. Dr. Amr Hussien Zaher

Professor of Reinforced Concrete Structure Structural Eng. Dept., Ain Shams University

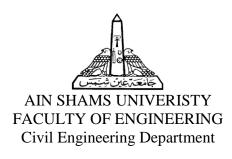
Dr. Mahmoud El kateb

Dr. Heba Mohamed Issa

Assistant Professor, Structural Eng. Dept., Ain Shams University

Assistant Professor, Concrete Structure Institute, Housing and Building Research Center

Cairo 2018



APPROVAL SHEET

Thesis : Master of Science in Civil Engineering (Structural). Student name: Nouran Mohamed Mahmoud Taha : Shear strengthening of reinforced concrete t-beams using anchored FRP wraps **Examiners Committee: Signature Prof. Dr. Mohamed Talat Moustafa** Professor of Concrete Structures, Structural Engineering Department , Faculty of Engineering, Cairo University, Egypt Prof. Dr. Ayman Hussein Hosny Professor of Concrete Structures, Structural Engineering Department Faculty of Engineering, Ain Shams University, Egypt

Date / /2018

Faculty of Engineering, Ain Shams University, Egypt

Prof. Dr. Amr Hussien ZaherProfessor of Concrete Structures,
Structural Engineering Department

STATEMENT

The thesis is submitted to Ain Shams University, Cairo, Egypt, in partial fulfilment of the requirements for the degree of Master of Science in Civil Engineering (Structural).

The Work included in this thesis was carried out by the author at Properties and Testing of materials Laboratory, Faculty of Engineering, Ain Shams University.

No part of this thesis has been submitted for a degree or qualification at any other university or institute.

Date : / / 2018

Name : Eng/Nouran Mohamed Taha

Signature :

Dedication

I dedicate this thesis to the people who devoted their lives for making me a successful person, my Father, my Mother, my Sister and my Brother. Gratitude and appreciation cannot describe my feelings towards them.

A special dedication to Dr.Amr Zaher, Dr. Mahmoud El kateb and Dr. Heba Issa who always encouraged and supported me.

Acknowledgement

First of all, utmost appreciation to the Almighty Allah for the divine intervention in this academic endeavor.

I am greatly privileged as this scientific effort was under the supervision of **Dr. Amr Zaher**. I'd like to express my profound gratitude and sincere appreciation for his continuous support and kind supervision.

I'd like to express my deepest gratitude to my teacher and supervisor **Dr. Heba Issa** for her great encouragement, endless flow of knowledge and great help where he always provides his students with her information and support.

Also I am really very much grateful to the family of the Properties and Testing of Materials Laboratory at Housing and Building Research Center for the technical support to finish my experimental program.

TABLE OF CONTENTS

TABLE OF CONTENTS	i
List of FIGURES	iv
List of Tables	vii
ABSTRACT	viii
Chapter 1	1
INTRODUCTION	1
1.1 General	1
1.2 Problem Statement	1
1.2.1 Necessity of Rehabilitation	1
1.2.2. CFRP Anchors in Shear implementations	2
1.3. Research Objectives	2
1.4. Scope of the Work	3
1.5. The Work Organization of Thesis	3
Chapter 2	5
BACKGROUND AND LITERATUE REVIEW	5
2.1. General	5
2.2. Shear Failure Modes	5
2.3. Strengthen RC Members Techniques	6
2.4. Previous research in strengthening of shear	7
2.4.1Wrapping System	7
2.4.2Space between FRP strip	8
2.5. Using FRP in engineering	8
2.5.1 FRP Material according to ACI	9
2.5.2 FRP Sheets	13
2.6 Characteristic of CFRP contrast with steel	14
2.6.1 Shear Strengthening Configuration	15
2.6.2 Variables effect on CFRP in Shear Strength	16
2.6.3 Size Effectiveness	17
2.7 Failure of FRP in Shear Strengthening	17
2.7.1 Failure at the FRP/Epoxy zone	18
2.7.2 Failure of the Concrete Substrate	18
2.7.3 FRP Rupture	21
2.8CFRP ANCHORS	23
2.8.1 Outline of studies on FRP Sheets Anchored with FRP Anch	ors23
2.8.2 Previous Research of Anchorage methods	25

2.7.3	Experiential Performance of FRP Sheets concentrated with FRP A	nchors28
2.9 Co	onstruction of FRP Anchors	35
Chapter	3	38
EXPERI	MENTAL PROGRAM	38
3.1 In	troduction	38
3.2 Te	est Specimens	38
2.3.1	Concrete Dimensions of Specimens	38
2.3.2	Design of Test Specimens	39
3.3 Sp	ecimens Formwork	40
3.4 Ma	aterial Properties	41
3.4.1	Concrete	41
3.4.2	Reinforcement	42
3.4.3	Fiber Reinforced Polymers	42
3.5 St	rengthening of Specimens	43
3.5.1	Surface preparation	43
3.5.2	CFRP anchor hole preparation	44
3.5.3	WET lay-up procedure	46
	reparation steps for epoxy resin	
3.4.3.2 A	pplying FRP wrappingInstall CFRP anchor	
	omenclature	
3.6.1	Strengthening Schemes	
	st Setup	
	easurement Devices	
	Linear Voltage Displacement Transducers	
3.8.2	Strain Gauges	
3.8.3	Data Logger	
_	4	
	S, ANALYSIS AND DISSCUSSION	
	oduction	
	Results	
	Control Beam (BC)	
	Specimen (BCU-100-NA)	
	pecimen (BCU-150-NA)	
	pecimen (BCU-200-NA)	
	pecimen (BCU-100-CAH)	
4265	necimen (RCU-100-CAI)	7Δ

4.2.7 Specimen (BCU-150-CAH)	77
2.7.8 Specimen (BCU-150-CAI)	80
2.7.9 Specimen (BCU-200-CAH)	84
2.4.10 Specimen (BCU-200-CAI)	87
4.2.11 Specimen (BCU-150-CSH)	91
4.3Analysis of the Results	94
4.3.1Effect of partial Wrapping for CFRP sheet with different spacing	94
4.3.2Effect of installation end anchors in web beam	96
4.3.3Effect of installation end anchors due to beam flange	99
4.3.4Effect of installation horizontal sheet at the end of wrapping	101
4.3.5Effect of partial Wrapping for CFRP sheet with 100 mm spacing	103
4.3.6Effect of partial Wrapping for CFRP sheet with 150 mm spacing	104
4.3.7Effect of partial Wrapping for CFRP sheet with 200 mm spacing	104
4.4 Crack Patterns and Modes of Failure	105
Chapter 5	112
SUMMARY AND CONCLUSION	112
5.1 Summary	112
5.2 Conclusions based on experimental	112
5.2.1 Effect of FRP Anchors	112
5.2.2 Effect of FRP in strengthening	114
5.3 Recommendations for Future work	114
REFERENCES	116

LIST OF FIGURES

Figure 2. 1: Types of shear failure (Williams, 2000)	6
Figure 2. 2: The repair/strengthening techniques for RC members (Flexure & shear)	
Figure 2. 3: Wrapping sketches in shear implementations	
Figure 2. 4: Failure mode of strengthening system	
Figure 2. 5: Various shapes of glass reinforcement	
Figure 2. 6: Fabricis Industrial product Component of woven carbon filaments [Wikipedia]	
Figure 2. 7: Aramid woven fibers [Wikipedia]	
Figure 2. 8: Various forms of fiber	
Figure 2. 9: CFRP sheet in one direction	
Figure 2. 10: Strength in two directions	
Figure 2. 11: Typical FRP and mild steel stress–strain curves	
Figure 2. 12: Shear strengthening planners	
Figure 2. 13: FRP wrapping arrangements	
Figure 2. 14: FRP debonding in the concrete substrate: (a) shear (b) flexure	
Figure 2. 15: Debonding areas for quarter bonded & u-wrapped FRP strips (Chen & Teng, 2003)	
Figure 2. 16: Shear crack passing FRP sheets & debonding	
Figure 2. 17:Debonding of FRP next cracking (Quinn, 2009)	
Figure 2. 18: shredded GFRP u-wrapped sheet	
Figure 2. 19: Shear crack crossing FRP strips (Quinn, 2009)	
Figure 2. 20: Classification of combined test setups used (Chen et al. 2001), renamed by Y	
	.24
Figure 2. 21: Elementary row (a) versus staggered (b) Anchor arrangements (Anil and Belg	
2010).	
Figure 2. 22: Three possible configurations of the threaded anchor rod system method	
(Deifalla &Ghobarah, 2006)	25
Figure 2. 24 : L-shaped form CFRP.	
Figure 2. 26: The mechanical fastener technique (Wu & Huang, 2008)	
Figure 2. 27: (a) GFRP anchors from (Eshwar, 2008); (b) CFRP anchor from (Niemitz,	.20
2008), (c) CFRP anchor from (Anil and Belgin, 2010), and (d) CFRPanchor from (Smith,	
2011)	29
Figure 2. 28 : CFRP anchors	
Figure 2. 30 : U-wrap with CFRP anchorage technique system	
Figure 2. 31: CFRP anchor holes previously and next making hole chamfer	
Figure 2. 32: Decrease in efficiency due to diameter and bend radius	
Figure 2. 33: FRP anchor pry-out with municipal concrete failure	
Figure 2. 34: FRP anchor pull-out failure (Zhang, et al., 2011)	
Figure 2. 35 : FRP anchor rupture	
Figure 2. 36:Commercially making CFRP and GFRP anchors	
Figure 2. 37: Dry anchor construction (Zhang, et al., 2011)	
Figure 2. 38: Wet anchor construction (Zhang, et al.,2011)	.37
Figure 3. 1 : Cross section of test specimens	.39
Figure 3. 2: Elevation of specimen reinforcement	
Figure 3. 3: Cross section reinforcement	
Figure 3. 4 : Formwork	
Figure 3. 5: Cross section of wood formwork as constructed	
Figure 3. 6 : Rounded to prevent a rupture of the CFRP	
A AMOND OF OF A CONTINUOUS OF PLOTOLIS IN LOPIUSED OF MIC OF IM CONTINUES CO	

Figure 3. 7: (a) Hole drilled into the concrete sample; (b) removing debris from the	
anchorage	45
Figure 3. 8: Drilled for and clarified anchorage hole	45
Figure 3. 9: Finished preparation of CFRP anchorage hole	45
Figure 3. 10: Preparation and mixing of epoxy	47
Figure 3. 11: Epoxy preparation on u shape	48
Figure 3. 12: Impregnating the CFRP strip while on the surface of the beam	49
Figure 3. 13: Wet anchor construction	
Figure 3. 14: Insertion install of the CFRP anchor	51
Figure 3. 15: Construction insulation of CFRP anchorage fan	52
Figure 3. 16: Finishing installation applying of CFRP anchors	52
Figure 3. 17: Pilot study T-section beam test setup	56
Figure 3. 18: Locations of LVDTs and rosette on specimens	58
Figure 3. 19: Locations of strain gauges on reinforcement bars	58
Figure 3. 20: Locations of strain gauges on FRP wraps	
Figure 3. 21: Data logger system	59
Figure 4. 1 : Load-deflection of control beam	61
Figure 4. 2 : Shear failure of control beam	
Figure 4. 3: Stirrup strain of control beam	
Figure 4. 4 : Failure mode of beam BCU-100-NA	
Figure 4. 5: CFRP debonding with and CFRP sheet rupture in beam BCU-100-NA	
Figure 4. 6: Load-deflection of beam BCU-100-NA	
Figure 4. 7: Steel rebar strain of BCU-100-NA	
Figure 4. 8: Stirrup strain of BCU-100-NA	
Figure 4. 9: Load-strain curves in 2nd sheet	
Figure 4. 10: Load-strain curves in 3rd sheet	
Figure 4. 11: Diagonal tension and CFRP debonding failure of beam BCU-150-NA(a,b).	
Figure 4. 12: Load-deflection of beam BCU-150-NA	
Figure 4. 13: Stirrup strain in BCU-150-NA	
Figure 4. 14: Load-strain curves in 2 nd sheet	
Figure 4. 15 : Failure mode of beam BCU-200-NA	
Figure 4. 16: Debonding in CFRP sheet with shear failure of beam BCU-200-NA	
Figure 4. 17: Load-deflection of beam BCU-200-NA	
Figure 4. 18: Stirrup strain of BCU-100-NA	
Figure 4. 19: Load-strain curves in second sheet	71
Figure 4. 20 : Failure mode of beam	
Figure 4. 21: Load-deflection of beam BCU-100-CAH	
Figure 4. 22 : Stirrup strain in BCU-100-CAH	
Figure 4. 23: Load-strain curves in 2 nd sheet	
Figure 4. 24: Failure modes of beam	76
Figure 4. 25 : Load-deflection of beam BCU-100-CAI	76
Figure 4. 26: Stirrup strain of BCU-100-CAI	
Figure 4. 27: Load-strain curves in 2 nd sheet	
Figure 4. 28: Failure modes of beam BCU-150-CAH	
Figure 4. 29 : Load vs. deflection of beam BCU-150-CAH	
Figure 4. 30 : Stirrup strain in BCU-150-CAH beam	
Figure 4. 31 : Load-strain curves in 2nd sheet	80
Figure 4. 32 : Failure mode of beam BCU-150-CAI	81
Figure 4. 33 : Load-deflection of beam BCU-150-CAI	

Figure 4. 34: Steel rebar strain in BCU-150-CAI	83
Figure 4. 35 : Stirrup strain of BCU-150-CAH	83
Figure 4. 36: Load-strain curves in 2 nd sheet	
Figure 4. 37: Failure mode of beam BCU-200-CAH	85
Figure 4. 38: Load-deflection of beam BCU-200-CAH	
Figure 4. 39: Stirrup strain in BCU-200-CAH	
Figure 4. 40: Load-strain curves in 2 nd sheet	
Figure 4. 41: Failure mode of beam	
Figure 4. 42: Load-deflection of beam BCU-200-CAI	
Figure 4. 43: Steel rebar strain in BCU-200-CAI	
Figure 4. 44: Stirrup strain in BCU-200-CAH beam	
Figure 4. 45: Load-strain curves in 2nd sheet	
Figure 4. 46: Beam BCU-150-CSH with horizontal sheet anchorage	
Figure 4. 47: Failure modes of beam	
Figure 4. 48: Load-deflection of beam BCU-150-CSH	
Figure 4. 49: Stirrup strain in BCU-150-CAH	
Figure 4. 50: Load-strain curves in 2nd sheet	
Figure 4. 51: Load-strain curves in horizontal sheet	
Figure 4. 52: Effect of wrap spacing on deflection behavior of T-section specimens with	
partial wrapping	
Figure 4. 53: The relation between Load and strain in CFRP sheets	96
Figure 4. 54: Effect of anchors on deflection behavior of T-section specimens with part	
wrapping	97
Figure 4. 55: Effect of horizontal anchor on strain in sheet of specimens with partial wr	apping
	98
Figure 4. 56: Effect of inclined anchor on strain in sheet of specimens with partial wrap	ping
	99
Figure 4. 57: Effect of inclined anchor on strain in sheet of specimens with partial wrap	ping
	101
Figure 4. 58: Effect of horizontal sheet on deflection of specimens	102
Figure 4. 59: Effect of horizontal sheet on strain in sheet of specimens with partial wrap	ping
vs. anchors	103
Figure 4. 60: Effect of wrapping in spacing 100 mm in deflection	104
Figure 4. 61: Effect of wrapping in spacing 150 mm in deflection	104
Figure 4. 62: Effect of wrapping in spacing 200 mm in deflection	
Figure 4. 63 : Failure of specimen (BC)	106
Figure 4. 64: Failure of specimen (BCU-1-00NA)	107
Figure 4. 65 : Failure of specimen (BCU-150-NA)	107
Figure 4. 66 : Failure of specimen (BCU-200-NA)	108
Figure 4. 67: Failure of specimen (BCU-100- CAH)	108
Figure 4. 68: Failure of specimen (BCU-100- CAI)	109
Figure 4. 69: Failure of specimen (BCU-150- CAH)	
Figure 4. 70 :Failure of specimen (BCU-150- CAI)	
Figure 4. 71 ;Failure of specimen (BCU-200- CAH)	
Figure 4. 72: Failure of specimen (BCU-200- CAI)	110
Figure 4 73: Failure of specimen (RCLI-150- CHS)	111

LIST OF TABLES

Table 2. 1: Properties of fibers	9
Table 3. 1 :Mix proportions by weight	41
Table 3. 2 : Resin properties	
Table 3. 3 :Fiber properties	
Table 3. 4: Appearance and physical properties of bonding materials	
Table 3. 5: Mechanical properties of bonding materials	
Table 3. 6: Experimental nomenclature used in this study	
Table 3. 7: Test specimen for T-section beams	
Table 4. 1 : Effect wrapping in deflection of Group A over control	95
Table 4. 2: Effect wrapping in deflection of Group B over control	
Table 4. 3: Effect wrapping in deflection of Group (C) over control	
Table 4. 4 :Effect wrapping in deflection of Group (D) over control	
Table 4. 5: Mode of failure of specimens	

ABSTRACT

Fiber-reinforced polymer (FRP) composites are used as repair and strengthening material for reinforced concrete (RC) members. The main purpose for use of FRP sheets is to debond when loads reach to the ultimate capacity this reason prompted researchers to investigate various approaches and designs increase the efficiency of FRP strengthening systems. Various types of anchor systems, wrapping techniques, and clamps have been searched to postpone and/or delay the debonding process which results in premature failure. FRP anchors are of particular advantage because it has the same material properties as the FRP sheets that are installed for strengthening or repair of the RC member.

This research studies and investigates the effectiveness of using commercially manufactured FRP anchors to secure FRP sheets installed to strengthen and repair RC beams in shear. Thirteen shear critical RC beams were strengthened in shear with U-wrapped FRP sheets. The test variables include the type of FRP sheets (CFRP), type of FRP anchors (GFRP), and spacing between U-wrapped FRP sheets. The number and the spacing of anchors placed inside the slab are many times more effective than those placed horizontally inside the web. Anchors of similar geometrical characteristics (e.g. embedment length) display similar effectiveness and strengthening configuration. The test results of the shear critical RC beams detected that the installation of FRP anchors are improved partially wrapping of FRP sheets on shear strengthening. Applied FRP anchors improved the shear strength by 15 % where compared to unanchored beams and delayed typical shear failure in beams. Using FRP anchors allowed the FRP sheets to increase their tensile capacity.

Additionally, as the width of a partially wrapping FRP sheet was increased; larger increase in strength was obtained when FRP anchors were used. The use of FRP anchors allowed the FRP sheets to develop their full tensile strength. Premature failure by CFRP debonding was not eliminated with the presence of FRP anchors; rather the critical failure zone was shifted from the bottom soffit of the slab to the concrete/steel rebar interface. The effective strain in the FRP sheet was predicted and compared with the experimental results.

The efficiency of FRP anchors were defined as the ratio of effective strain in the FRP sheet with and without anchors and was related to the increase in strength in beams. A good correlation was established between the FRP anchor efficiency and the increase in strength. A step-by-step FRP anchor installation procedure was developed and a model to predict the number of FRP anchors required to secure FRP sheet was proposed. This is the most comprehensive examination of beams strengthened with FRP sheets and FRP anchors. This study provides the engineers engineer with the basic understanding of the behaviour and failure modes of beams strengthened with FRP sheets and anchors

Keywords: *CFRP*; anchors; *T-section*; *Strengthening*; *Beams*; *shear*; *Wrapping*; *sheets*.

Chapter 1 INTRODUCTION

1.1 General

Strengthening or rehabilitation using CFRP (Carbon Fiber Reinforced Polymers) is a convenient material for RC structures because of its light-weight, flexibility and easy installation techniques. Epoxy is used to make full bonding between concrete and FRP.

1.2 Problem Statement

CFRP, in case was fails in debonding of the sheet, it prevents the increase of the ideal capacity of the material, which leads to precocious failure. CFRP anchors provides a solution to many types of failure. This research concentrates on studying the behavior of using CFRP sheets and anchors on shear strengthening of reinforced concrete beams.

1.2.1 Necessity of Rehabilitation

Restoration and strengthening techniques are very important and needs. Whereas commutation or demolishing may be a main and constant solution. In many conditions, destroying old structures cost such that rehabilitation could be an applicable solution. These constructions are necessary to rehabilitated when they approach the end of their service life. Bridges and structures require restoration and repair from excessive size, traffic loads, and damages resulting from the corrosion of reinforcing steel (Noel & Soudki, 2011).

The promotion of fiber-reinforced polymer (FRP) composites as a repair and strengthening material for reinforced concrete (RC) beams, slabs and columns in structural engineering has increased during the previous twenty years (Cao, et al., 2005; Triantfillou &Antonopoulos, 2000; Bank, 2006; American Concrete