

# AIN SHAMS UNIVERSITY

### FACULTY OF ENGINEERING

Structural Engineering

# Restoration of Bond Strength for Fire-Damaged RC Elements

A Thesis submitted in partial fulfilment of the requirements of the degree of

Master of Science in Civil Engineering

(Structural Engineering)

by

### Nesma Abd EL-Hameed Mohammed Ghazaly

Bachelor of Science in Civil Engineering

(Structural Engineering)

The Higher Institute of Engineering, El Shorouk Academy, 2013

Supervised By

### Prof. Dr. Omar Aly Elnawawy

Professor of Reinforced Concrete Structures Structural Engineering Department Faculty of Engineering-Ain Shams University

#### **Dr. Ahmed Rashad Mohamed**

Assistant Professor Structural Engineering Dept. Faculty of Engineering Ain Shams University

### Dr. Mohamed Kohail Mohamed

Assistant Professor Structural Engineering Dept. Faculty of Engineering Ain Shams University

Cairo - (2018)



**Thesis** : Master of Science in Civil Engineering (Structural)

**Researcher Name:** Nesma Abdel Hameed Mohammed Ghazaly

**Thesis Title** : Restoration of Bond Strength for Fire Damaged RC

Elements

<b>Examiners Committee</b>	<b>Signature</b>
Prof. Dr. Gouda Mohamed Ghanem Professor of Properties and Testing of Materials Faculty of Engineering - Helwan University	
Prof. Dr. Ayman Hussein Khalil Professor of Reinforced concrete structures Structural Engineering Department Faculty of Engineering - Ain Shams University	••••••
Prof. Dr. Omar Aly Elnawawy Professor of Reinforced concrete structures Structural Engineering Department Faculty of Engineering - Ain Shams University (Supervisor)	



**Thesis** : Master of Science in Civil Engineering (Structural)

Researcher Name: Nesma Abdel Hameed Mohammed Ghazaly

**Thesis Title** : Restoration of Bond Strength for Fire Damaged RC

Elements

<b>Supervision Committee</b>	<b>Signature</b>
Prof. Dr. Omar Aly Elnawawy Professor of Reinforced concrete structures Structural Engineering Department	
Faculty of Engineering - Ain Shams University  Dr. Ahmed Rashad Mohamed  Assistant Professor  Structural Engineering Department  Faculty of Engineering - Ain Shams University	
Dr. Mohamed Kohail M. Fayez Assistant Professor Structural Engineering Department Faculty of Engineering - Ain Shams University	••••••

# **Statement**

This thesis is submitted as a partial fulfillment of Master of Science in Civil Engineering (Structural Engineering), Faculty of Engineering, Ain shams University. The author carried out the work included in this thesis, and no part of it has been submitted for a degree or a qualification at any other scientific entity.

.

Student name
Nesma Ghazaly
Signature
Date:

## Researcher Data

Name : Nesma Abdel Hameed Ghazaly

Date of birth : 05 December 1991

Place of birth : Cairo, Egypt

Last academic degree : Bachelor of Science

Field of specialization : Structural Engineering

University issued the degree : El Shorouk Academy

Date of issued degree : June 2013

Current job : Structural Designer Engineer.

#### **ABSTRACT**

Reinforced concrete structures are vulnerable to high temperature conditions such as those during exposure to fire. The exposure of reinforced concrete (RC) structural elements to high temperature leads to loss in the mechanical properties of concrete and most importantly loss of bond between concrete and steel rebars. Recovering the structural integrity of heat-damaged RC elements requires extensive repair and rehabilitation works.

This thesis contains details of an experimental and analytical study conducted to evaluate the residual bond strength between concrete and steel rebars after subjected to elevated temperature and to investigate the effective repairing materials and techniques in restoring bond strength for heat –damaged concrete elements. The beam-end specimens were tested in three conditions, in ambient temperature, after subjected to elevated temperature of 800 °C, or after subjected to 600 °C. Steel fiber reinforced concrete and concrete were used as the repairing materials. Shallow and deep repair techniques were used as repairing techniques. The tested rebar was embedded in two concrete covers 30 and 50 mm. The embedded lengths 5Ø and 8Ø were conducted in this study.

The experimental results showed that the residual bond strength for the heated specimens was almost 26% and 32% at 800°C and 600°C respectively, of the original ultimate bond strength with significant increase in slip corresponding to the ultimate bond stress, especially when exposed to 800°C. The residual, restored and original bond strengths decreased with the increase of the bonded length from 5Ø to 8Ø. Although, they increased with the increase of the concrete cover. Considering the repair of heat-damaged specimens, the deep and shallow repair techniques using concrete restored almost 87% and 84% respectively, of the ultimate

bond strength. However, the restored bond strength almost 94% and 90% respectively, of the original ultimate bond strength for specimens repaired using steel fiber reinforced concrete. There is no difference in the restored bond strength for both repairing techniques for specimens exposed to ambient temperature. The effective repairing material used in this study, the steel fiber reinforced concrete (SF), it restored 96% and 91% of the original bond strength of specimens with 50 mm concrete cover.

To predict the bond stress – slip behavior for all specimens simple relationships are proposed based on experimental data and different parameters studied. The modified model for bond stress – slip curve, has been proposed by different researchers is suitable for reproduction of the ascending branch of the bond – slip curve. Key analytical results showed that the proposed equations give a good prediction of the bond strength for different specimens. Also, a good agreement between the modified proposed analytical model for bond stress- slip curve and the experimental data can be observed.

**Keywords:** Bond strength; Elevated temperature; Steel fiber; Deep repair; Bonded length; Heat damaged; Restoration.

#### ACKNOWLEDGEMENT

First and Foremost praise to Allah, the Almighty, the greatest of all, on whom ultimately we depend for sustenance and guidance. His continuous grace and mercy was with me throughout my life and ever more during the tenure of my research.

I would like to express my deepest thanks and appreciation to my supervisor, **Dr. Omar El-Nawawy**, for his continuous support, valuable guidance, and for giving me the opportunity to investigate such an interesting point of research.

Special thanks for my supervisors, **Dr. Ahmed Rashad, and Dr. Mohamed Kohail** for their continuous support, valuable assistance and providing the guidance necessary to complete this research.

I would like to thank the technical staff and labors of the Laboratory of Properties and Testing of Materials at Ain Shams University for their hard work during the experimental phase of this research. Also, I would like to thank the labors of the gas furnace for their hard work.

Eventually, I would like to express my warmest and sincerest heartfelt gratitude and appreciation to all my family especially my father and beloved mother who stood beside me and supported me in every step in my life. I dedicate this thesis to my mother for her assistance, support, encouragement, and patience especially during hard times and without her, I would not have been able to accomplish this work.

### **PUBLICATIONS**

N. Ghazaly, O. Nawawy, A. Rahad, and M. Kohail, (2018) 'Evaluation of bond strength between steel rebars and concrete for heat-damaged and repaired beam-end specimens '"Engineering Structures. Elsevier Ltd, 175, pp. 661–668.doi: 10.1016/j.engstruct.2018.08.056.

## TABLE OF CONTENTS

		Page
AB	STRACT	i
AC	KNOWLEDGEMENT	iii
Pui	BLICATION	iv
TA	BLE OF CONTENTS	V
Lis	T OF FIGURES	viii
Lis	T OF TABLES	xii
No	TATION	xiii
Сн	APTER (1):INTRODUCTION	1
1.1	Background	. 1
1.2	Research Objectives	2 3
1.3	Scope	3
1.4	Thesis Outline	4 5 5
Сн	APTER (2): LITERATURE REVIEW	5
2.1	Introduction	
2.2	Bond Behavior at ambient temperature	5
2.3	Bond Behavior After Exposure to Elevated Temperature	13
2.4	Assessment of Heat Damaged Concrete Structures	20
	2.4.1 The First Methodology	20
	2.4.2 The Second Methodology Involves Three steps	22
2.5	Bond Testing	25
	2.5.1 Pullout Test	25
	2.5.2 Beam End Test	26
	2.5.3 Beam Anchorage Test	27
2.6	Repair Procedures	28
	2.6.1 Removal of Concrete	28
	2.6.2 Replacement of Weakened Reinforcement	30
	2.6.3 Partial Replacement of Concrete	31
2.7	Repairing Techniques for Heat- Damaged Concrete	22
	Elements	33
	2.7.1 Deep and Shallow Repair Techniques.	33
	2.7.2 Concrete Jacketing	33
	2.7.3 Fiber Reinforced Polymer (FRP)	34

2.8 Analytical Study for Prediction of the Bond Strength	36
2.8.1 Predicting the Bond Strength at Ambient Temperature	36
2.8.2 Predicting the Bond Strength after Heat Exposure	39
2.8.3 Predicting the Bond Strength for Repaired Specimens	41
2.8.4 Bond Stress-Slip Relationships	41
2.9 Needed Research	42
CHAPTER (3): RESEARCH PLAN, CONSTRUCTION, AND TESTING	44
PROCEDURES	7-7
3.1 Introduction	44
3.2 Research Plan	44
3.2.1 Objective	44
3.3 Experimental Program	45
3.3.1 Test Matrix	45
3.3.2 Specimens Geometry and Steel Reinforcement Detailing	47
3.3.3 Specimens Fabrication	49
3.4 Materials Properties	51
3.4.1 Cement	51
3.4.2 Coarse Aggregates	51
3.4.3 Fine Aggregates	52
3.4.4 Super Plasticizer	52
3.4.5 Mixing of Concrete	52
3.4.6 Reinforcing Steel	53
3.5 Repairing Materials	54
3.6 Repairing Techniques	56
3.7 Heating and Cooling Regimes	58
3.8 Testing of Specimens	59
CHAPTER (4): TEST RESULTS & DISCUSSIONS AND ANALYTICAL	61
STUDY	
4.1 Introduction	61
4.2 Specimens Exposed to Ambient Temperature 4.2.1 General	61 61
4.2.1 General 4.2.2 Failure Modes	61
	62
4.2.3 Bond Stress-Slippage Curves	64
4.3 Specimens Exposed to Elevated Temperature 4.3.2 General	64
4.3.1 Damage Due to Elevated Temperature	64
4.3.3 Failure Modes	65
4.3.4 Bond Stress-Slippage Curves	65
Done Sulve Suppuge Car too	00

4.4 Specimens Repaired using Deep Technique	68
4.4.1 General	68
4.4.2 Failure Modes	68
4.4.3 Bond Stress-Slippage Curves	69
4.5 Specimens Repaired using Shallow Technique	73
4.5.1 General	73
4.5.2 Failure Modes	73
4.5.3 Bond Stress-Slip Curves	74
4.6 Discussion of Test Results	77
4.6.1 Analysis of Test Results	77
4.6.1 The Effect of Elevated Temperature	78
4.6.2 The Effect of Concrete Cover	81
4.6.3 The Effect of Bonded Length	82
4.6.4 The Effect of Repairing Technique and Repairing	84
Material	04
4.7 Analytical Study	87
4.7.1 Proposed Equation for Specimens at Ambient	87
Temperature	07
4.7.2 Proposed Equation for Heat-Damaged Specimens	89
4.7.3 Proposed Equation for Repaired Specimens	89
4.7.4 Bond Stress-Slippage Relationship	91
CHAPTER (5): SUMMARY, CONCLUSIONS, AND	0.7
RECOMMENDATIONS	95
5.1 Summary	95
5.2 Conclusions	96
5.3 Recommendations for Further Studies	97
References	98

## LIST OF FIGURES

		Page
Fig. (1.1)	Depth of Breaking Off for Deep Repair	2
Fig. (1.2)	Depth of Breaking Off for Shallow Repair	2
Fig. (2.1)	Bond Force Transfer Mechanism	6
	Force Components Parallel and Perpendicular to the	7
	Steel Concrete Interface	7
Fig. (2.2b)	Shear Stress Distribution in XY Plane of Concrete	7
Fig. (2.3)	Effect of Bonded Length on the Bond Behavior	8
Fig. (2.4)	Effect of the Rebar Type on the Bond Behavior	9
Fig. (2.5)	Effect of the Loading Rate on the Bond Behavior	9
Fig. (2.6)	Pull out Specimen	10
Fig. (2.7)	Relationship of Bond Stress Ratio and Concrete	11
Fig. (2.8)	Double Pull out Specimen	12
Fig.(2.9)	Influence of the Rebar Size on the Average Bond	12
Fig.(2.10)	Influence of the Rebar Type	13
Fig.(2.11)	Heating-Cooling Regime in Furnace	14
Fig.(2.12)	Relative Compressive Strength of Concretes	15
	Relative Pullout Load Ratio at 40 mm Concrete Cover	15
•	Relative Pullout Load Ratio at 70 mm Concrete Cover	15
	Variation in Bond Strength and Temperature for	1.0
,	150 mm Embedded Length	16
Fig.(2.14b)	Variation in Bond Strength and Temperature for	17
	300 mm Embedded Length	17
Fig.(2.15)	_	17
	Temperature	17
Fig.(2.16)	Residual Strength of Heated Stressed Dense	10
	Aggregate Concrete after Cooling	18
Fig.(2.17)	Residual Compressive Strength of Concrete	10
	after Cooling	18
Fig.(2.18)	Test Setup	19
	Heating Curve	19
Fig.(2.20)	Development of Peak Free End Slippage with	20
	Temperature	20
Fig. (2.21)	Deterioration of Compressive Strength and Bond	20
, ,	Strength after heat Exposure	20
Fig.(2.22)	Color Changes in Concrete	23
	Fire Damage Factors for Columns, Beams and Slabs	24
= ' '	Based on Temperature Profiles	24
Fig.(2.24)	Schematic of the Pullout Test	25

Fig.(2.25)	Schematic of the Beam End Test	26
Fig.(2.26)	Specimen Geometry and Reinforcement	27
Fig.(2.27)	Bond Beam Test – National Bureau of Standard Bond Beam Test	27
Fig.(2.28)		28
Fig.(2.29)	<u> </u>	
6 ( > )	Equipment	29
Fig.(2.30)	• •	29
Fig.(2.31)		31
Fig.(2.32)		32
Fig.(2.33)	- ·	33
Fig.(2.34)	Concrete Jacketing before Applying the New Concrete	34
Fig.(2.35)		35
Fig.(2.36)		35
-	T12 NWC Specimens	37
•	T12 LWC Specimens	37
Fig.(2.39)	•	
11g.(2.39)	and the original CEB-FIP Model Code 1990	38
Fig.(2.40)	_	
1 15.(2.40)	Bond Strength	39
Fig.(2.41)	_	
115.(2.71)	Compressive Strength Relationship and Experimental	40
	Results Database	10
Fig.(2.42)	Analytical Versus Experimental Bond – Slip Curves.	42
1 18 (21 12)	That year versus Emperimental Bond Sup Carves	
Fig.(3.1a)	Specimens with Concrete Cover 50 mm	46
Fig.(3.1b)	Specimens with Concrete Cover 30 mm	46
Fig.(3.2)	Configuration of the Beam End Specimens.	48
Fig.(3.3)	Regions of Bond Breaker	48
Fig.(3.4)	Reinforcement Cage in Mold.	49
Fig.(3.5)	Test Specimen on Vibrating Table	50
Fig.(3.6)	The Concrete Surface Leveling using Trowel.	50
Fig.(3.7)	Curing of Specimens and Cubes	50
Fig.(3.8)	Concrete Mixer.	53
Fig.(3.9)	Corrugated Round Steel Fibers	54
Fig.(3.10)	Indirect Tension Test.	55
Fig.(3.11)	Deep Repair Technique	56
Fig.(3.12)	Applying Shallow Repair Technique	57
Fig.(3.13)	Repair Techniques	57
Fig.(3.14)	Applying Bonding Agent	58
Fig.(3.15)	Applying Repairing Material	58

Fig.(3.16)	Gas Furnace and Heating Regimes	59
Fig.(3.17)	A Schematic Diagram of the Test Set up	60
Fig.(3.18)	Test set up	60
Fig.(4.1)	Typical Failure Modes for Specimens Exposed to Ambient Temperature	62
Fig.(4.2)	Bond Stress - Slippage Curve for C30-5Ø-A	62
Fig.(4.3)	Bond Stress - Slippage Curve for C30-8Ø-A	63
Fig.(4.4)	Bond Stress - Slippage Curve for C50-5Ø-A	63
Fig.(4.5)	Corner Spalling on Cubes After Heat Exposure	64
-	View of Spalled and Discolored for Heat- Damaged	64
Fig.(4.7)	Typical Failure Mode for Heat - Damaged Specimens	65
Fig.(4.8)	Bond Stress - Slippage Curve for C30-5Ø-H <sub>800</sub> Specimen	66
Fig.(4.9)	Bond Stress - Slippage Curve for C30-5Ø-H <sub>600</sub> Specimen	66
Fig.(4.10)	Bond Stress - Slippage Curve for C30-8Ø-H <sub>800</sub> Specimen	67
Fig.(4.11)	Bond Stress - Slippage Curve for C50-5Ø-H <sub>800</sub> Specimen	67
Fig.(4.12)	Typical Failure Mode for Deep Repaired Specimens	68
Fig.(4.13)	Bond Stress - Slippage Curve for C30-5Ø-A-DC	69
	Specimen	0)
Fig.(4.14)	Bond Stress - Slippage Curve for C30-5Ø-H <sub>800-</sub> DC Specimen	69
Fig.(4.15)	Bond Stress - Slippage Curve for C30-5Ø-A-DSF Specimen	70
Fig.(4.16)	Bond Stress - Slippage Curve for C30-5Ø-H <sub>800</sub> -DSF Specimen	70
Fig.(4.17)	Bond Stress - Slippage Curve for C30-8Ø-A-DSF Specimen	71
Fig.(4.18)	Bond Stress - Slippage Curve for C30-8Ø-H <sub>800</sub> -DSF Specimen	71
Fig.(4.19)	Bond Stress - Slippage Curve for C50-5Ø-A-DSF Specimen	72
Fig.(4.20)	Bond Stress - Slippage Curve for C50-5Ø-H <sub>800</sub> -DSF Specimen	72
Fig (4.21)	Typical Failure Mode for Deep Repaired Specimens	73
_	Bond Stress - Slippage Curve for C30-5Ø-A-SC	
115.(1.22)	Specimen Supplies Sup	74
Fig.(4.23)	Bond Stress - Slippage Curve for C30-5Ø-H <sub>800</sub> -SC Specimen	74
Fig.(4.24)	Bond Stress - Slippage Curve for C30-5Ø-A-SSF Specimen	75
Fig.(4.25)	Bond Stress - Slippage Curve for C30-5Ø-H <sub>800</sub> -DSF Specimen	75