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EVALUATION OF DOLIME FINE PERFORMANCE IN MITIGATING THE EFFECTS OF AN EXPANSIVE SOIL

By

Aymen Hasan Naser

A Thesis Submitted to the Faculty of Engineering at Cairo University in Partial Fulfillment of the Requirements for the Degree of

MASTER OF SCIENCE

In

CIVIL ENGINEERING - PUBLIC WORKS

FACULTY OF ENGINEERING, CAIRO UNIVERSITY GIZA, EGYPT 2022

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Under the Supervision of

Prof. Dr. Ahmed Hisham Mohammed Dr. Shehab Sherif Wissa

Professor of Geotechnical Engineering and Foundations Public Works Dept.

Cairo University

Assist. Professor of Geotechnical

Engineering and Foundations Public Works Dept.

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Approved by the Examining Committee

Prof. Dr. Ahmed Hisham Mohammed

Assoc. Prof. Dr. Asmaa Moddather Hassan

Prof. Dr. Sayed Mohammed Elsayed

(Professor at the Faculty of Engineering, Ain Shams University, Egypt)

FACULTY OF ENGINEERING, CAIRO UNIVERSITY GIZA, EGYPT 2022 **Engineer's Name:** Aymen Hasan Naser

Date of Birth: 17/04/1994

Nationality: Iraqi

E-mail: <u>aymanh1188@gmail.com</u>

Phone: (+20)1013598553, (+964)7703105016

Address: Cairo – Haram

Registration Data: 01/03/2017

Awarding Date: / /2022

Degree: Master of Science

Department: Civil Engineering-Public Works

Supervisors:

Prof. Dr. Ahmed Hisham Mohammed

Dr. Shehab Sherif Wissa

Examiners:

Prof. Dr. Ahmed Hisham Mohammed
Assoc. Prof. Dr. Asmaa Moddather Hassan
Prof. Dr. Sayed Mohammed Elsayed
(Professor at the Faculty of Engineering, Ain Shams University, Egypt)

Title of Thesis:

EVALUATION OF DOLIME FINE PERFORMANCE IN MITIGATING THE EFFECTS OF AN EXPANSIVE SOIL.

Key Words:

Swelling soil, Swelling pressure, Dolime fine, Bentonite, Clay characterization.

Summary:

Expansive clays are considered problematic soils as they exhibit significant volume changes with the change in moisture content. Mixing swelling clays with dolime fine has been used as an improvement technique. Dolomite chips are obtained by crushing dolomite stones; while crushing a substantial solid waste is produced, it is called dolime fine. Dolime fine has very high percentage of CaO. It has been recommended as a binding agent that can replace pure lime. So dolime fines can be utilized to stabilize expansive soil. Their strength and availability characterize dolomite chips. An industrial swelling soil has been prepared by mixing bentonite with a natural clay soil to use it in this study, the mixing proportion was 75% Bentonite and 25% natural clay soil. Dolime chips were brought from Erbil city (northern Iraq), while bentonite was brought from Samawa city (southern Iraq), as natural soil was brought from the marshes of Basra city (southern Iraq).

Samples were prepared in the laboratory by mixing the swelling soil with (0%, 4%, 8%, and 12%) of the dolime fine relative to the weight of the soil for 0, 14, 28 curing days.

The test results showed that the addition of dolime fine, in different percentages, reduces the MDD, SP, L.L and I_p. Simultaneously, the MOC, CBR, UCS, and P.L were increased with increasing the percentage of dolime fine and the curing period.



Disclaimer

I hereby declare that this thesis is my own original work and that no part of it has been submitted for a degree qualification at any other university or institute.

I further declare that I have appropriately acknowledged all sources used and have cited them in the references section.

Name: Aymen Hasan Naser Date...../2022

Signature:

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A Activity of Clay

ASTM American Standard for Testing and Materials

C Clay Content

CBR California Bearing Ratio

D Diameter

e_o Initial Void Ratio

F.S. Free Swell

γ Unit Weight

G_s Specific Gravity

γ_d Dry Unit Weight

γd⋅max. Maximum Dry Unit Weight

MDD Maximum Dry Density

GSD Grain Size Distribution

L.L Liquid Limit

P.L Plasticity Limit

S.L Shrinkage Limit

I_p Plasticity Index

OMC Optimum Moisture Content

P Load

P_{sw} Swelling Pressure

S Swelling Percent

SP Swelling Potential

UCS Unconfined Compressive Strength

W_c Water content