

شبكة المعلومات الجامعية التوثيق الإلكتروني والميكروفيلو

بسم الله الرحمن الرحيم





MONA MAGHRABY



شبكة المعلومات الجامعية التوثيق الإلكتروني والميكروفيلو



شبكة المعلومات الجامعية التوثيق الالكتروني والميكروفيلم



MONA MAGHRABY



شبكة المعلومات الجامعية التوثيق الإلكترونى والميكروفيلم

جامعة عين شمس التوثيق الإلكتروني والميكروفيلم قسم

نقسم بالله العظيم أن المادة التي تم توثيقها وتسجيلها علي هذه الأقراص المدمجة قد أعدت دون أية تغيرات



يجب أن

تحفظ هذه الأقراص المدمجة بعيدا عن الغبار



MONA MAGHRABY





ANALYSIS OF LOW STRENGTH RC BEAMS STRENGTHENED WITH FRP SHEETS

By

Abdul Rahman Mamoun Ali Mohamed

A Thesis submitted to the
Faculty of Engineering, Cairo University
In partial fulfillment of the
Requirements for the Degree of
MASTER OF SCIENCE
In
Structural Engineering

FACULTY OF ENGINEERING, CAIRO UNIVERSITY GIZA, EGYPT 2021

ANALYSIS OF LOW STRENGTH RC BEAMS STRENGTHENED WITH FRP SHEETS

By **Abdul Rahman Mamoun Ali Mohamed**

A Thesis Submitted to the
Faculty of Engineering, Cairo University
In partial fulfillment of the
Requirements for the Degree of
MASTER OF SCIENCE
In
Structural Engineering

Under the Supervision of

Prof. Dr. Hany Ahmed Abdalla

Dr. Ahmed Mohammed Youssef

Professor of Concrete Structures Faculty of Engineering, Cairo University Assistant Professor of Concrete Structures Faculty of Engineering, Cairo University

FACULTY OF ENGINEERING, CAIRO UNIVERSITY GIZA, EGYPT 2021

ANALYSIS OF LOW STRENGTH RC BEAMS STRENGTHENED WITH FRP SHEETS

By **Abdul Rahman Mamoun Ali Mohamed**

A Thesis Submitted to the Faculty of Engineering, Cairo University In partial fulfillment of the Requirements for the Degree of MASTER OF SCIENCE

In
Structural Engineering

Approved by the

Examining Committee

Prof. Dr. Hany Ahmed Abdalla Thesis Main Advisor

Prof. of concrete structure – Cairo University

Prof. Dr. Mohamed Talaat MostafaInternal Examiner

Prof. of Concrete Structure – Cairo University

Prof. Dr. Gouda Mohamed Ghanem External Examiner

Prof. of Concrete Structure – Helwan University

FACULTY OF ENGINEERING, CAIRO UNIVERSITY GIZA, EGYPT 2021 **Engineer's Name:** Abdul Rahman Mamoun Ali

Date of Birth: 15 / 05 / 1992 **Nationality:** Egyptian

E-mail: eng_abdelrahman920@yahoo.com

Phone: 01122666692

Address: 66 Elshahid Mustafa Kamal-Qena

Registration Date:1 / 10 / 2017Awarding Date:..../...........Degree:Master of ScienceDepartment:Structural Engineering

Supervisors:

Prof. Hany Ahmed Abdalla

Dr. Ahmed Mohammed Youssef

Examiners:

Prof. Gouda Mohamed Ghanem (External examiner)

Prof. of Concrete Structure – Helwan University Prof. Mohamed Talaat Mostafa (Internal examiner) Prof. Hany Ahmed Abdalla (Thesis main advisor)

Title of Thesis:

Analysis of low strength RC beams strengthened with FRP sheets

Key Words:

Shear behavior; flexural behavior; low strength concrete; CFRP sheets; finite element method by ANSYS

Summary:

Carbon Fiber Reinforced Polymers (CFRP) sheets offer an attractive solution to enhance shear and flexural capacities of RC beams. The main objective of this research is to analyze the behavior of low strength concrete beams strengthened with CFRP. The nonlinear finite element program ANSYS was used to conduct the analysis of this research. The results of the program were verified against experimental results available in the literature. The parameters considered in this study include concrete strength, number of CFRP layers, and angle of inclination of the strips on the longitudinal axis of the beam. The results show that CFRP strips could effectively enhance the flexural and shear behavior of RC beams depending on their initial condition. The concrete strength was found to be a dominant factor affecting the improving rate of shear and flexural capacities of strengthened beams. The results of this study are compared to those estimated according to the Egyptian and American codes for advanced composite materials.

Disclaimer

I hereby declare that this thesis is my own original work and that no part of it has been submitted for a degree qualification at any other university or institute.

I further declare that I have appropriately acknowledged all sources used and have cited them in the references section.

Name: Abdul Rahman Mamoun Ali Date: / / 2021

Signature:

Dedication

To my parents, my brother, my sisters, and all my friends.

Acknowledgments

I sincerely thank my advisor Prof .Dr. Hany Ahmed Abdalla as he positively believed in my work. It would not be possible for me to accomplish any of my research goals without his constructive and helpful pieces of advice and academic support.

I would like to thank Dr. Ahmed Mohammed Youssef for his guidance, advice, support and valuable discussion review while working on my thesis, and his great effort to achieve my objectives.

I also would like to thank my father, my mother, my sisters, my brother and all my friends who always help me to achieve success. I would not achieve this aim without their help and support.

Table of Contents

Table of	Contents	iv
List of T	`ables	vi
List of F	igures	vii
	: 1 : Introduction	1
1.1.	General	
1.2.	Research objectives	
1.3.	Scope of Research	
1.4.	Contents of the Thesis	
-	2 : Literature Review	
2.1.	Introduction	
2.2.	Literature review	
2.2.1.	6 6	
2.2.2.	Experimental tests for RC beams strengthened with CFRP in flexure	
2.2.3.	Experimental tests for RC beams strengthened with CFRP in shear	10
2.2.4.	Nonlinear finite element modeling for RC beams strengthened with CFF	RУ
in flex	ture	12
2.2.5.	Nonlinear finite element modeling for RC beams strengthened with CFF	RР
in she	ar	
2.2.6.		
2.2.0.	Summary	
	: 3 : Finite Element Modeling	
3.1.	Main assumptions for flexural and shear model	
3.2.	Selection of element type in ANSYS for flexural and shear model	
3.2.1.	·	
3.2.2.		
		24
3.2.3.	Carbon Fiber Reinforced Polymer CFRP and steel plates (Loading and	2.4
	rting)	
3.3.	Real constants	
3.4.	Defining material properties	
		27
	1.1. Linear isotropic material	
3.4.	1.2. Multilinear isotropic material	27
3.4.	1.3. Concrete material	29
3.4.2.	Steel reinforcement	30
3.4.3.	Loading and supporting steel plates	31
3.4.4.	Carbon Fiber Reinforced Polymer (CFRP)	
3.4.5.	Epoxy bond material	
3.4.6.	Defining model geometry	
	•	33
3.4.7.	Loading, Supporting, and ANSYS Solution Control for non-linear	20
•	sis	
-	4: Validation of Finite Element Model	
4.1.	Flexural analysis model	41

4.1.1.	Comparison between FE results and the Egyptian Code	46
4.2.	The shear model	50
4.2.1.	Comparison between FE results and the Egyptian Code	55
Chapter	r 5 : Parametric Study	60
5.1.	Introduction	60
5.1.1.	Parameters considered for flexural strengthening	60
5.1.2.	Parameters considered for shear strengthening	60
5.2.	The ANSYS Finite Element results	63
5.2.1.	The first group of specimens (Flexural Model)	63
5.2.2.	The second group of specimens (Flexural Model)	69
5.2.3.	The third group of specimens (Flexural Model)	79
5.2.4.	Comparison between finite element results of the flexural model and	1
Egypt	ian code for composites (ECP No. 208-2005):	88
5.2.5.	_	
5.2.6.		
5.2.7.		
compo	osites (ECP No. 208-2005):	99
5.3.	Summary	
Chapter	r 6 :Summary, Conclusions and Recommendations for Future Stud	ies101
6.1.	Summary	101
6.2.	Conclusions	101
Reference	ces	103

List of Tables

Table 2.1 : Properties of test specimens [4]	4
Table 2.2: Beam data and capacity [5]	5
Table 2.3: Details of test specimens [7]	
Table 2.4: Details of strengthened test specimens [9]	.11
Table 2.5: Details of strengthened test specimens [14]	.22
Table 3.1: Selection of element type in ANSYS	.25
Table 3.2(a): Real constants for steel reinforcement and concrete in ANSYS (Flexura	1
Model)	.26
Table 3.2(b): Real constants for steel reinforcement and concrete in ANSYS (shear	
model)	
Table 3.3(a): Steel material properties for the flexural model in ANSYS	.30
Table 3.3(b): Steel material properties for the shear model in ANSYS	
Table 3.4: Dimension of flexural beam model in ANSYS	.33
Table 3.5: Dimension of shear beam model in ANSYS	
Table 4.1: Details of the used flexural model for verification with [8]	.41
Table 4.2: Experimental, FE model and Egyptian code results	
Table 4.3: Details of the used shear model for verification with [7]	
Table 4.4: The results of the experimental and FE model and Egyptian code	.59
Table 5.1 (a): The specimens used for the flexural model first group	.61
Table 5.1 (b): The specimens used for the flexural model second group	.61
Table 5.1 (c): The specimens used for the flexural model third group	.62
Table 5.2 (a): The specimens used for the shear model fourth group	.62
Table 5.2 (b): The specimens used for the shear model fifth group	.62
Table 5.3: The results of the flexural model first group	
Table 5.4: The results of the flexural model in the second group	
Table 5.5: Results of the third group (flexural model)	.80
Table 5.6: Comparison between ultimate load capacity of FE analysis and ECP No.	
208-2005 in flexural for beams with one layer of CFRP	.88
Table 5.7: Comparison between ultimate load capacity of FE model and ECP No. 208	
2005 in flexural for beams with two-layer CFRP	.89
Table 5.8: The results of the shear model fourth group	.89
Table 5.9: The results of the shear model fifth group	
Table 5.11: Comparison between ultimate load capacity of FE model and ECP No. 20	
2005 in shear for beams with one layer CFRP 45°1	.00

List of Figures

Figure 2.1: Cross-section of beams before and after strengthen [4]	4
Figure 2.2: (a) Control beam (b) Type 1 (c) Type 2 [5]	5
Figure 2.3: Four point flexural test [7]	7
Figure 2.4: Experimental and analytical results of CFRP that is 76mm wide, and 1.4	
mm thick [7]	7
Figure 2.5: Experimental and analytical results of CFRP that is 102mm wide, and	
4.78mm thick [7]	
Figure 2.6: Experimental and analytical results of the beams bonded with CFRP Fab.	ric
[7]	
Figure 2.7: Dimension of the tested model [8]	9
Figure 2.8: Control beam under static and cyclic load [8]	9
Figure 2.9: The results of control beam, beam strengthened with single layer and two	
layer CFRP [8]	10
Figure 2.10: Control specimens details [9]	10
Figure 2.11: Strengthened specimens details (a) [9]	
Figure 2.11: Strengthened specimens details (b) [9]	
Figure 2.12: Comparison of the results with ACI440 [9]	
Figure 2.13: Comparison of results with ACI440 [9]	
Figure 2.14 (a): FE model dimension [10]	
Figure 2.14 (b): FE model element [10]	
Figure 2.15: Comparison between the FE & experimental results of control beam	
[10]	13
Figure 2.16: Comparison between the FE & experimental results of control beam with	
one layer CFRP [10]	
Figure 2.17: Comparison between the FE & experimental results of control beam with	
one layer BFRP [10]	
Figure 2.18: Comparison between the FE & experimental results of control beam with	
hybrid combined one layer CFRP & one layer BFRP [10]	
Figure 2.19: Details of shear model [11]	
Figure 2.20: half beam FE model [11]	
Figure 2.21: Comparison between the strengthened beams results in the FE &	
experimental model [11]	16
Figure 2.22: Comparison between the control and strengthened beam results in the F	
model [11]	
Figure 2.23: Details of beams with variable reinforcement ratio [12]	17
Figure 2.24: The variation of results when reinforcement ratio changed [12]	
Figure 2.25: Comparison between experimental and FE model results [12]	
Figure 2.26: Experimental tested specimen's details [13]	
Figure 2.27: The results of the study [13]	
Figure 2.28: Comparison between the non-damaged & damaged (50%, 100%)	
specimens [13]	21
Figure 2.29: Details of tested specimens [14]	
Figure 2.30: Details of strengthened tested specimens [14]	
Figure 3.1: Solid 65 geometry [18]	
Figure 3.2: Link 180 geometry [18]	
Figure 3.3: Solid 185 Homogeneous structural solid geometry [18]	

Figure 3.4: Modeling of steel reinforcement by duplicating surface area	25
Figure 3.5: Modified Hognestad mathematical model [16]	
Figure 3.6: Stress-strain curve concrete (10 MPa) by modified Hognestad model	
Figure 3.7: Stress-strain curve concrete (15 MPa) by modified Hognestad model	
Figure 3.8: Stress-strain curve concrete (20 MPa) by modified Hognestad model	
Figure 3.9: Stress-strain curve concrete (25 MPa) by modified Hognestad model	
Figure 3.10: Stress-strain curve concrete (20 MPa) by modified Hognestad model	
Figure 3.11: Orthotropic material (wood fiber)	
Figure 3.12(a): CFRP material properties details for the flexural model in ANSYS	
Figure 3.12(a). CFRP material properties details for the shear model in ANSYS	
Figure 3.13: Set up of RC beam flexural test [8]	
Figure 3.14: Duplicated area used for drawing steel reinforcement bar in ANSYS	
Figure 3.15: Flexural beam model in ANSYS	
Figure 3.16: Flexural beam model after meshing 25*25mm in ANSYS	
Figure 3.17: Flexural beam model after meshing with a wire frame in ANSYS	
Figure 3.18: Flexural beam model steel reinforcement with a wire frame in ANSYS	
Figure 3.19: Flexural beam model Epoxy and side view with a wire frame in ANSYS	
Figure 3.20: A set up of RC beam shear test [7]	
Figure 3.21: A set up of strengthened RC beam shear test [7]	
Figure 3.22: The shear beam model in ANSYS	
Figure 3.23: The shear beam model after meshing 25*25mm in ANSYS	
Figure 3.24: The shear beam model after meshing with a wire frame in ANSYS	
Figure 3.25: The shear beam model steel reinforcement with a wire frame in ANSYS	
Figure 3.26: The shear beam model CFRP, Epoxy, and side view with a wire frame in	
ANSYS	
Figure 3.27: New work plane for the direction of fibers for CFRP in ANSYS	
Figure 3.28: Flexural beam model loading in ANSYS	
Figure 3.29: Shear beam model loading in ANSYS	
Figure 3.30: Hinged support in ANSYS	39
Figure 3.31(a): Solution control in ANSYS	
Figure 3.31(b): Solution control in ANSYS	
Figure 3.31(c): Solution control in ANSYS	
Figure 4.1: Deformed shape for control beam (mid-point) in ANSYS	41
Figure 4.2: Cracks in control beam by ANSYS v14.5	
Figure 4.3: Cracks in beam strengthened with one layer CFRP by ANSYS v14.5	42
Figure 4.4: Cracks in beam strengthened with two-layer CFRP by ANSYS v14.5	
Figure 4.5: Failure pattern in control beam [8]	43
Figure 4.6: Failure pattern in beam strengthened with one layer CFRP [8]	43
Figure 4.7: Relation between load (kN) and deflection (mm) by ANSYS	44
Figure 4.8: Comparison between control and strengthened beams [8]	44
Figure 4.9: Comparison between the experimental and FE results (Control Beam)	45
Figure 4.10: Comparison between the experimental and the FE model (Beam with or	ne-
layer CFRP)	
Figure 4.11: Comparison between the experimental and the FE model (Beam with tw	
layer CFRP)	
Figure 4.12: stress-strain curve for FRP [17]	
Figure 4.13: Stress and strain distribution in the rectangular section subjected to	-
ultimate flexural moment [17]	48
Figure 4.14: The moment in the simply supported beam according to the four-point	
load system	49
<i>y</i>	